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US EPA RECORDS CENTER REGION 5



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DRIFT-PLATTEVILLE AQUIFER

SOURCE AND GRADIENT CONTROL WELLS

CONSTRUCTION AND AQUIFER TEST REPORT

ERT

A RESOURCE ENGINEERING COMPANY

**DRIFT-PLATTEVILLE AQUIFER
SOURCE AND GRADIENT CONTROL WELLS
CONSTRUCTION AND AQUIFER TEST REPORT**

Submitted to

**U.S. Environmental Protection Agency, Region V
and
Minnesota Pollution Control Agency**

Submitted by

**Reilly Tar & Chemical Corporation
Indianapolis, Indiana**

Pursuant to

**Remedial Action Plan Section Numbers 9.1.2 and 9.2.2
Exhibit A to the Consent Decree in
United States of America, et al. v. Reilly Tar & Chemical Corp., et al.
U.S. District Court, District of Minnesota, Civil No. 4-80-469**

December 9, 1987

INTRODUCTION

Construction and testing have been completed for the Drift-Platteville Aquifer Source and Gradient Control Wells at the Reilly Tar & Chemical Corporation N.P.L. Site in St. Louis Park, Minnesota in accordance with Work Plans submitted by Reilly per the Consent Decree-Remedial Action Plan (RAP). This report presents the logs for three new well installations, the results of pumping tests performed at each new well, and field adjustments to the approved designs for each well. This document fulfills the reporting requirements of RAP Sections 9.1.2 and 9.2.2.

Approvals for the Drift Platteville Aquifer Source Control Well Work Plan and the Drift-Platteville Gradient Control Well Work Plan were received on July 9, 1987. The last permits required for construction were received on August 11, 1987; and, in accordance with RAP Sections 9.1.2 and 9.2.2, all construction, testing, and reporting have been completed within 120 days of that date (December 9, 1987). Construction included the installation of the three wells, construction of appropriate well houses, and connections to the sanitary sewers. Pumping tests included monitoring water levels in the Drift-Platteville Aquifer in numerous observation wells during periods of stressed and non-stressed aquifer conditions.

WELL CONSTRUCTION

The Drift-Platteville Aquifer Source and Gradient Control Wells were drilled and installed in accordance with the approved, amended Work Plans. There were no field adjustments to the approved designs of each well. Bergerson-Caswell, Inc. constructed each well under the direction of ERT, Inc. The Water Well Record for each well is presented in Appendix A, along with the pilot soil boring logs and grain size analyses. As indicated on the Water Well Records, direct rotary techniques were used to advance a nominal 10-inch diameter hole through the drift. Six-inch well screen and/or pipe was then set in the hole and grouted into place. A graded sand pack was placed around the screens in the two Drift Aquifer wells. The Platteville Aquifer well was completed by drilling a nominal six-inch diameter hole through the full thickness of the Platteville Limestone.

Upon completion of the wells, each well was developed. The Drift Aquifer Source Control Well was developed for approximately 32 hours, using a combination of air-lifting and high-velocity jetting. This well responded nicely to the development by producing a sediment-

free discharge with an approximate specific yield of 10 gpm per foot of drawdown during development.

The Platteville Aquifer Source Control Well was developed by air-lifting. A sediment free discharge was obtained after four hours with an approximately specific yield of only two gpm per foot of drawdown.

The Drift-Platteville Aquifer Gradient control Well was developed by airlifting and high-velocity jetting. A total of 42 hours was spent developing this well. The well was slow to respond to development, although there were no fine grained silts or clays encountered in the pilot soil boring or the well itself that could have accounted for the slow development. At the end of development the well was producing a sediment-free discharge with an approximate specific capacity of 7 gpm per foot of drawdown.

The measuring point elevants for each well were surveyed following completion of each well, with the following results:

Drift Aquifer Source control Well:	895.83 Feet MSL
Platteville Aquifer Source Control Well:	895.79 Feet MSL
Drift-Platteville Aquifer Gradient Control Well:	908.18 Feet MSL

DRIFT AQUIFER SOURCE CONTROL WELL AQUIFER TEST

The Drift Aquifer Source Control Well aquifer test was the first of three aquifer tests performed. In preparation for the three tests, potential observation wells in the area were located and examined. Wells that were not sealed, damaged, or destroyed were subjected to a brief slug test to demonstrate the well's response to hydraulic stress. The following wells recovered to 90 percent of their original water level after the removal of one well volume of water, and were considered for use as observation wells during the aquifer tests:

P8	P124	W20
P11	P201	W22
P14	P203	W26
P15	W2	W100
P69	W5	W143
P109	W9	W-1*
P110	W11	W-2*
P112	W17	NL-1*
P121	W18	MW-1*
P123	W19	MW-2*

* These are monitoring wells at the National Lead/Taracorp site.

The following wells were not considered for use as observation wells during the aquifer tests because they were either damaged or they did not respond sufficiently during slug tests: W27, W13, W12, W132, P9, P45, P65, P25, P118, and P122.

The observation wells selected for use during the Drift Aquifer Source Control Well aquifer test were wells W2, W5, W9, W11, W18, P14, P15, P124, P203, Taracorp wells MW-1 and MW-2, and the Platteville Aquifer Source Control Well. This array of wells is shown in Figure 1, and includes at least one Drift Aquifer observation well within 200 feet, and one Drift Aquifer observation well within 1000 feet, of the pumping well, as indicated in the Work Plan. Well W2 was used to identify extraneous influences (e.g., water level changes due to precipitation) because it is beyond the cone of influence of the pumping well.

In-Situ, Inc. Hermit data loggers were used to record water level data for wells W2 and W5 for this test, and an In-Situ, Inc. SE-200 system was used for the other 11 wells. Water levels were recorded for 48 hours prior to the pumping phase of the aquifer test. The Drift Aquifer Source Control Well was pumped at 200 gallons per minute commencing on September 17, 1987 at 7:47 p.m., and ending on September 20, 1987 at 10:37 p.m. (approximately 75 hours). Water levels were then measured for 48 hours during which time the levels recovered to pre-test conditions. Data was lost from Taracorp well MW-2 during the pumping phase of the aquifer test due to an interruption in the transducer circuit.

Many of the observation wells exhibited water level fluctuations in response to the aquifer test. Five of the eight observation wells in the Drift formation produced data amenable to analysis (P14, P15, W9, P124, P203). The data from these wells were analyzed using the Prickett method for analysis of pumping test data from an unconfined aquifer (Prickett, 1965). The main assumptions inherent in the Prickett method are as follows:

- 1) aquifer is homogeneous, isotropic, of uniform thickness and has infinite areal extent,
- 2) pumping well fully penetrates unconfined aquifer, implying horizontal flow,
- 3) flow to the pumping well is not steady state and
- 4) well storage is neglected.

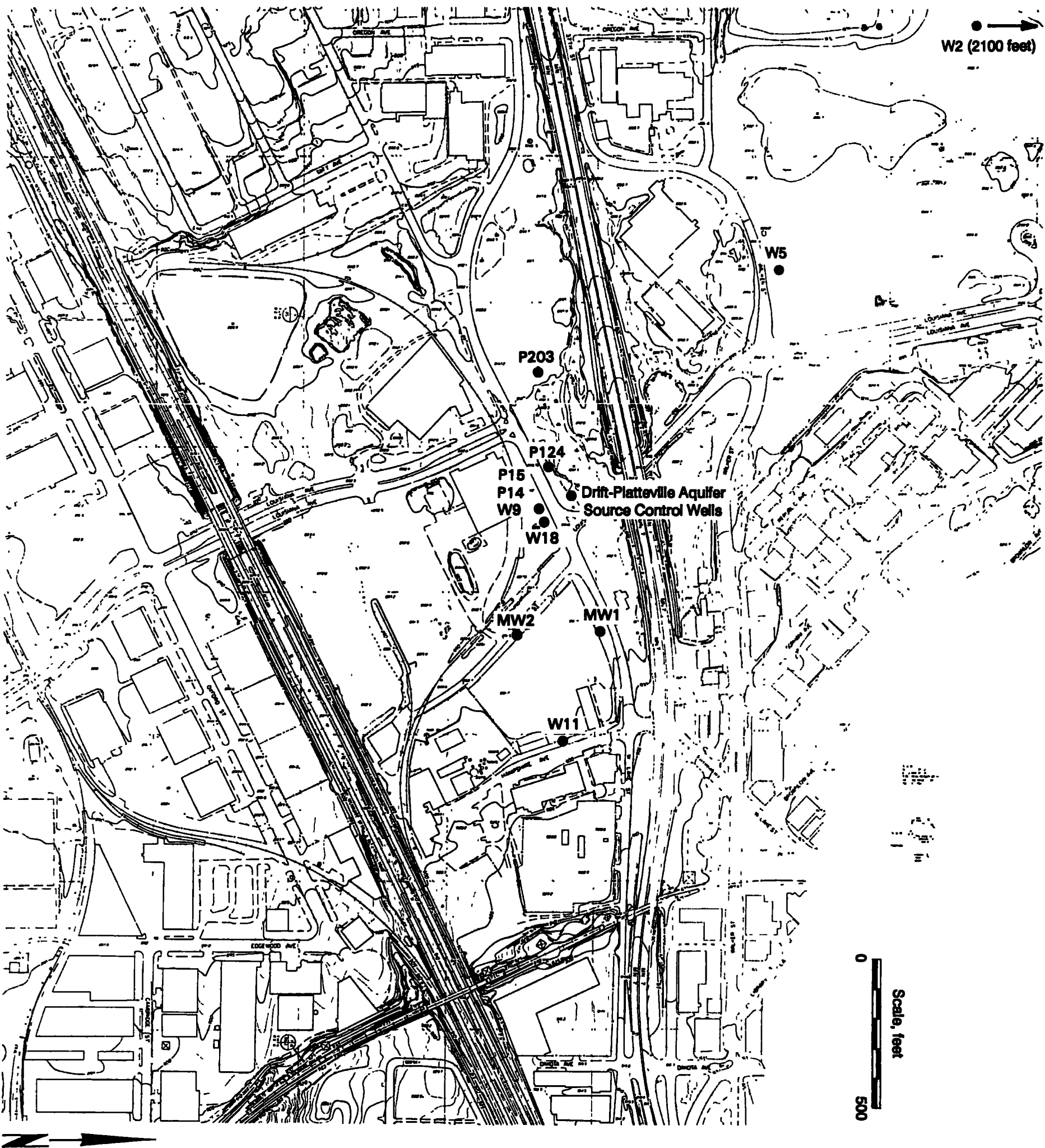


Figure 1.
Location of Wells for the Drift Aquifer
Source Control Well Aquifer Test

Hantush (1964) suggests that if the pumping well is only partially penetrating, drawdown in wells within 1.5 times the thickness of the aquifer may need to be corrected because the assumption of primarily horizontal flow could be violated. The correction is:

$$s' = s - s^2/2d$$

where:

s is the original drawdown

d is the thickness of the aquifer and

s' is the corrected drawdown.

For the first pump test, wells P14, P15 and W9 are within 1.5 times d, assuming d = 60 feet and the radial distance for each of the wells is 75 feet. The maximum drawdown in these three wells was 1.8 feet, producing a corrected drawdown of 1.77 feet. Because the maximum correction is so small, the analyses were done without correcting for partial penetration.

The data and the matchpoints used in the analyses for two representative wells (P15 and P124) are shown in Figures 2 and 3. The values calculated for the transmissivity for the first pump test range from $6.2e+4$ gpd/ft to $1.7e+5$ gpd/ft (Table 1), with a mean value of $1.0e+5$ gpd/ft. These values are similar to those reported in the literature (CDM, 1985; Stark and Hult, 1984; Hult and Schoenburg, 1984; Hickok, 1981; and Barr, 1977). The values calculated for storativity range from $2.5e-2$ to $4.5e-1$ (Table 1), with a mean value of $1.4e-1$. These are typical values for an unconfined aquifer.

The prepumping data for each of the wells analyzed do not show any appreciable trends, although the transducer in well W9 did show some instability with time, both in the background data and the pump test data. For this reason, only the early time data from well W9 were analyzed. The background well data (W2) did not show any appreciable changes over the duration of the first pump test (Figure 4) and were not corrected for in the analyses.

The barometric pressure data and precipitation data (collected from the Minneapolis-St. Paul Airport) over the duration of the three pump tests is presented in Figure 5. The background well data for all three pump tests do not show any response to the variations in these parameters and therefore the pump test data were not corrected for the effects of precipitation or variations in barometric pressure.

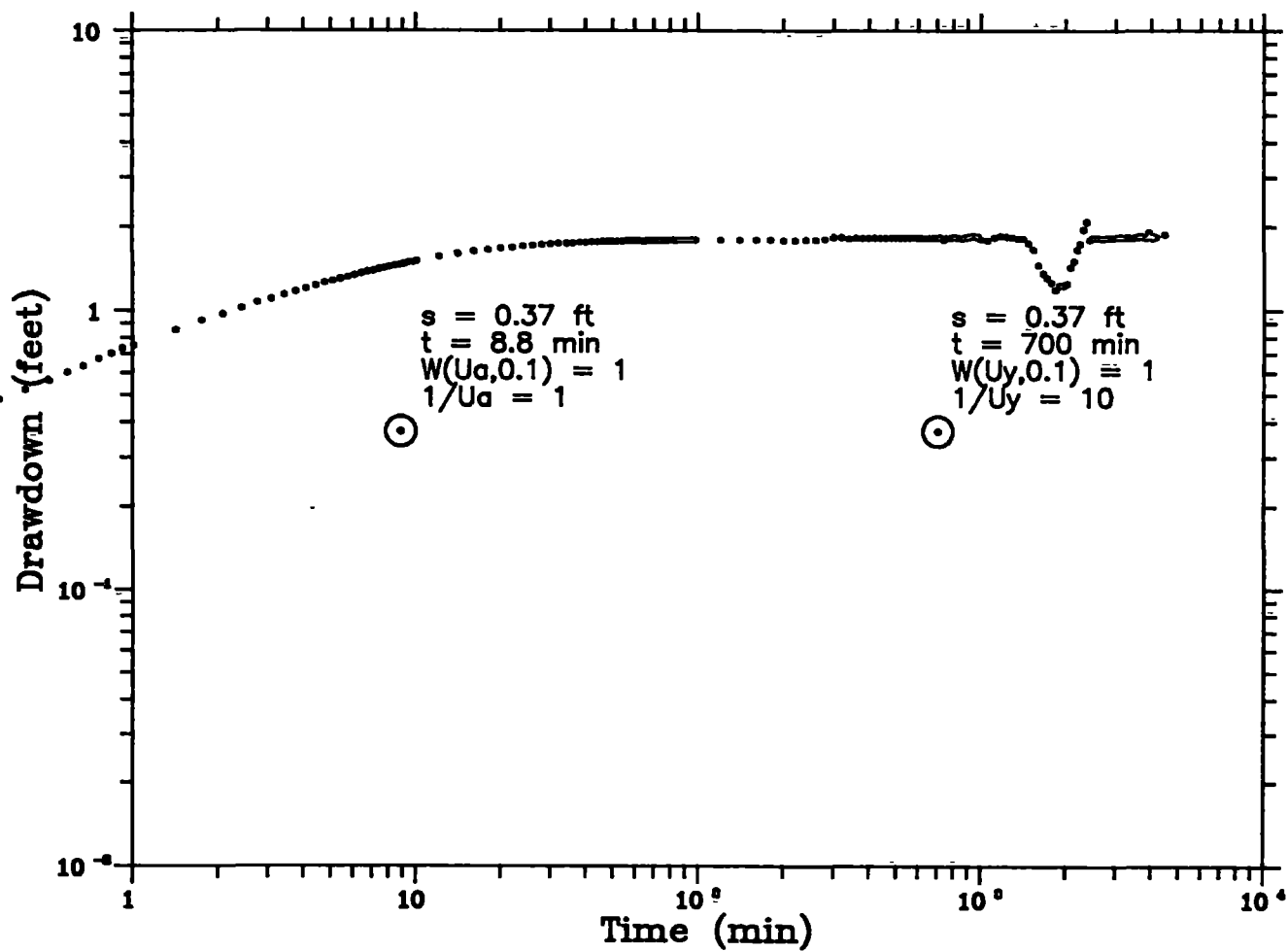


Figure 2. Drift Aquifer Source Control Well Aquifer Test

Drawdown Curve for Observation Well P15

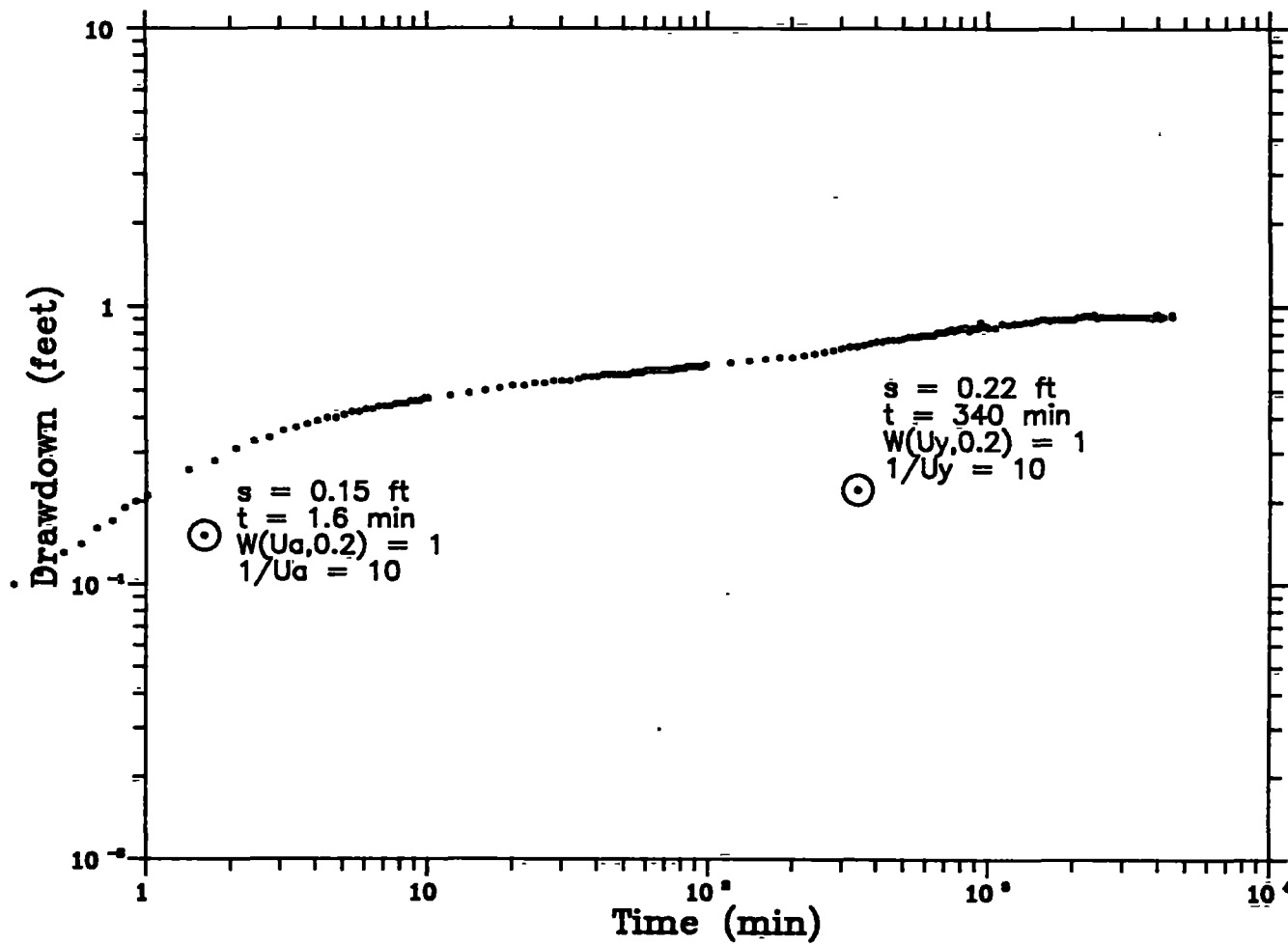


Figure 3. Drift Aquifer Source Control Well Aquifer Test,

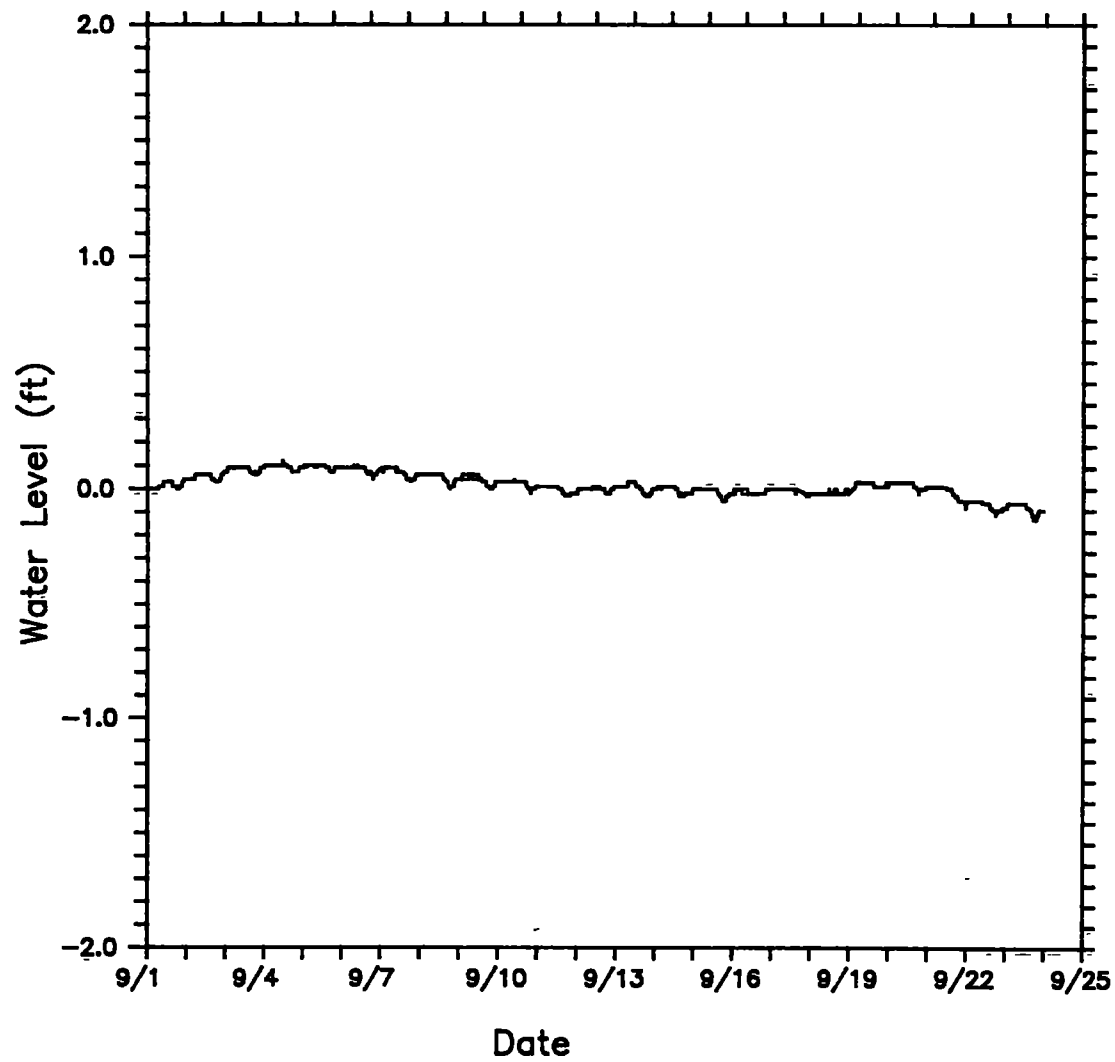
Drawdown Curve for Observation Well P124

TABLE 1
TRANSMISSIVITY VALUES FROM THE
DRIFT AQUIFER SOURCE CONTROL WELL AQUIFER TEST

<u>WELL NO.</u>	<u>TRANSMISSIVITY (GPD/FT)</u>	<u>STORATIVITY</u>
P14	7.7×10^4	1.47×10^{-1}
P15	6.2×10^4	2.86×10^{-1}
W9	7.2×10^4	3.83×10^{-2}
P124	1.3×10^5	2.51×10^{-2}
P203	1.7×10^5	1.9×10^{-1}

Average T = 1.0×10^5 gpd/ft

Average S = 1.37×10^{-1}



**Figure 4: Drift Aquifer Source Control Well Aquifer Test,
Background Water Level Data - Observation Well W2**

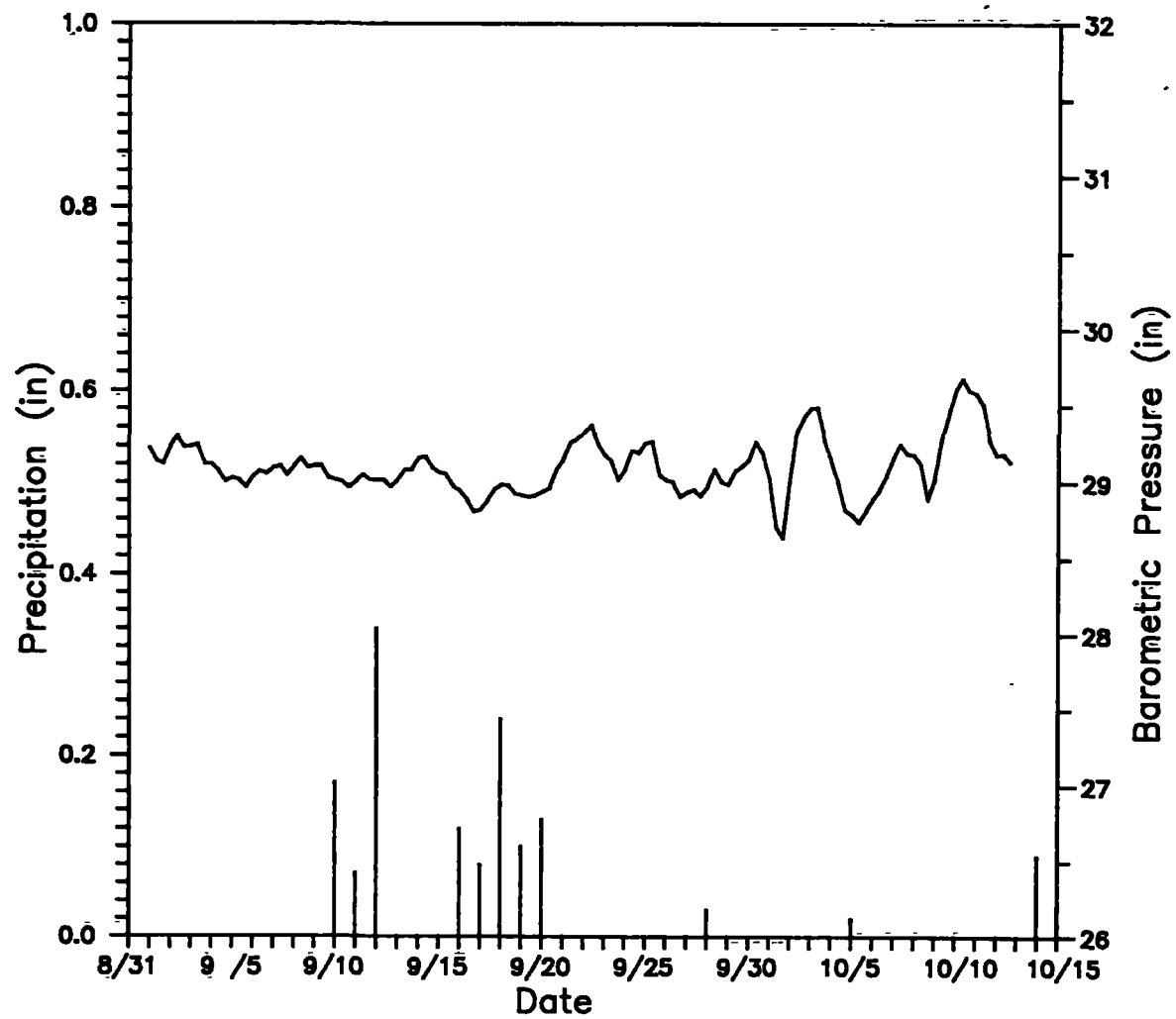


Figure 5. Precipitation and Barometric Data

PLATTEVILLE AQUIFER SOURCE CONTROL WELL AQUIFER TEST

The observation well selected for use during the Platteville Aquifer Source Control Well aquifer test were wells W18, W19, W20, W22, W26, W100, P14, P15, P121, P124, P203, Taracorp well MW-1, and the Drift Aquifer Source Control Well. These wells are shown in Figure 6. As indicated in the Work Plan, the array of observation wells for this aquifer test includes at least one Platteville Aquifer observation well within 300 feet, and one Platteville Aquifer observation well at about 1000 feet, from the pumping well. Well W100 was used to identify extraneous influences (e.g., water level changes due to precipitation) because it is beyond the cone of influence of the pumping well.

In-Situ, Inc. Hermit data loggers were used to record water levels in wells W100 and W20, and an In-Situ, Inc. SE-200 system was used for the other dozen wells. Water levels were recorded for 48 hours prior to the pumping phase of the aquifer test. The Platteville Aquifer Source Control Well was pumped at 55 gallons per minute commencing on September 26, 1987 at 10:08 p.m., and ending on September 27, 1987 at 10:20 p.m. (approximately 24 hours). Higher pumping rates caused too much drawdown in the well. Water levels were then measured for 41 hours during which time the levels recovered to pre-test conditions. Data was lost from Well W-22 during the recovery phase of the aquifer test due to an interruption in the transducer circuit.

Many of the observation wells exhibited water level fluctuations in response to the aquifer test. Five of the six Platteville observation wells produced data amenable to analysis (W18, W19, W22, W26, P121). Because the Platteville has been shown to be hydraulically linked with the Drift (Hult and Schoenburg, 1984), the data from the second pump test were analyzed using the Walton method for semi-confined aquifers (Walton, 1962). The main assumptions inherent to the method are as follows:

- 1) aquifer is homogeneous, isotropic, of uniform thickness and infinite areal extent,
- 2) pumping well fully penetrates the semi-confined aquifer,
- 3) overlying the aquitard is an unconfined aquifer,
- 4) water removed from storage is discharged instantaneously with decline of head and,
- 5) well storage is neglected.

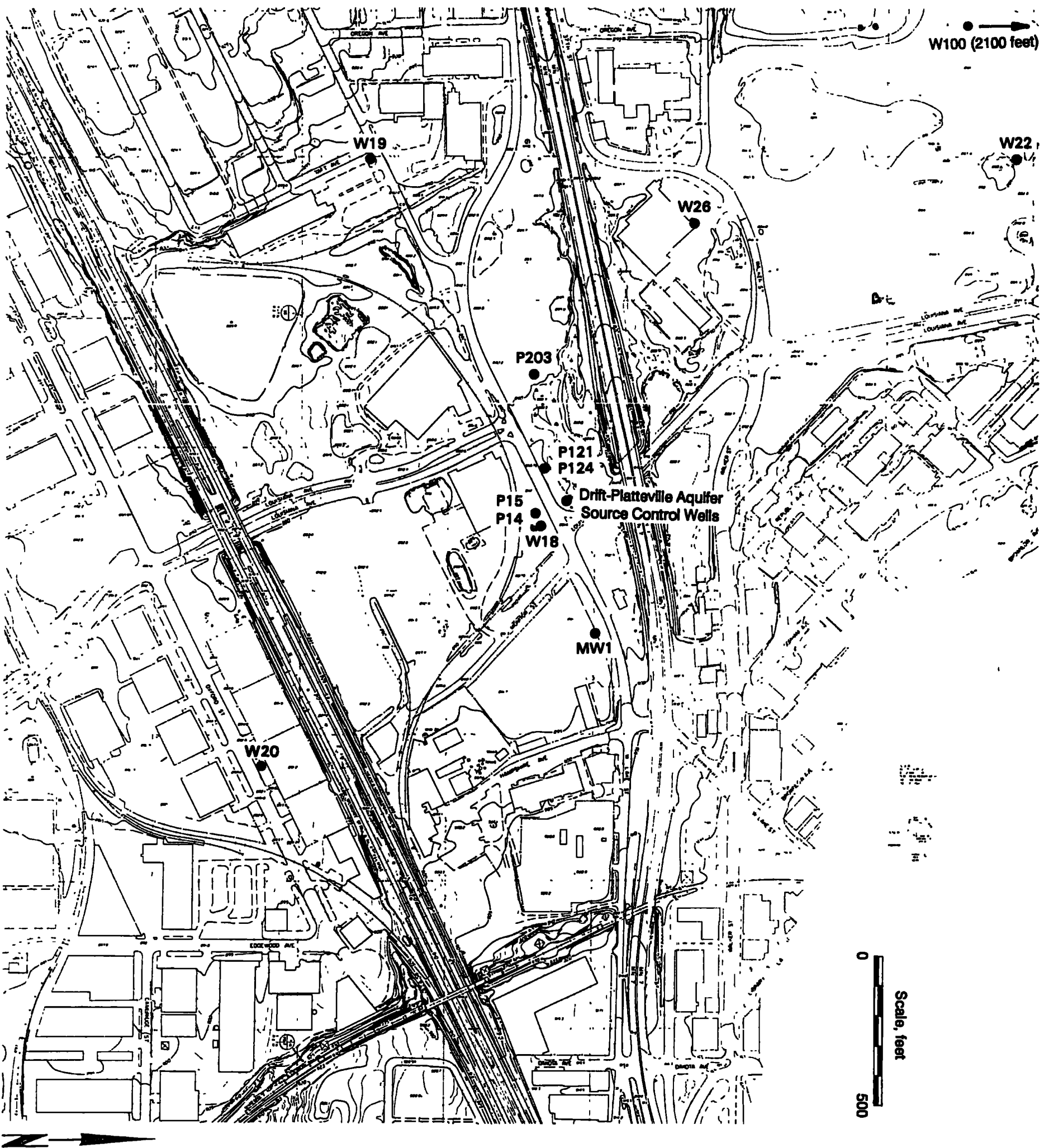


Figure 6. Location of Wells for the Platteville Aquifer Source Control Well Aquifer Test

The data matchpoints used in the analyses are shown in Figures 7 and 8 for two representative wells (W18, W19). The values calculated for transmissivity for the second pump test range from $2.0e+4$ gpd/ft to $4.4e+4$ gpd/ft (Table 2), with a mean transmissivity of $2.8e$ gpd/ft. These values fall within the range reported in the literature of $2.32e+4$ (Hult, 1981) to $6.7e+4$ gpd/ft (Stark and Hult, 1984) for the Platteville.

The prepumping data for each of the wells analyzed did not show any appreciable trends. However, the transducers in wells P121 and W26 both exhibited instabilities with increasing time. For this reason, only the early time data from these wells were used in the analyses.

The background well data (W100) did not show any appreciable changes over the period of the second pump test (Figure 9) and were not corrected for in the analyses.

The results from the first two pump tests suggest that the Platteville and Drift aquifers are hydraulically connected. During the Drift source control pump test, several of the observation wells screened in the Platteville showed noticeable drawdown, and wells P14 (Drift) and W18 (Platteville) exhibited responses that are typical of mutually leaky aquifers (Hantush, 1967). During the Platteville source control pump test, the response of the observation wells in the Platteville was typical of a semi-confined aquifer. The observation wells in the Drift did not show as great a response to the pumping of the Platteville, but this is partially due to the difference in pumping rates during the first two tests.

DRIFT-PLATTEVILLE GRADIENT CONTROL WELL AQUIFER TEST

The observation wells selected for use during the Drift-Platteville Aquifer Gradient Control Well aquifer test were wells W2, W11, W17, W143, P109, P110, P112, Taracorp wells W-1, W-2, MW-1, MW-2, and NL2. This array of wells is shown in Figure 10, and it includes at least one Drift Aquifer observation well within 600 feet, and at least one Drift Aquifer observation well within 1000 feet, of the pumping well as required by the Work Plan. Well W2 was used to identify extraneous influences (e.g., water level changes due to precipitation) because it is beyond the cone of influence of the pumping well.

In-Situ, Inc. Hermit data loggers were used to record water levels at wells P109, P110, and P112. The background well (W2) was monitored manually using an electric water level tape throughout the aquifer test. An In-Situ, Inc. SE-200 system was used to monitor water

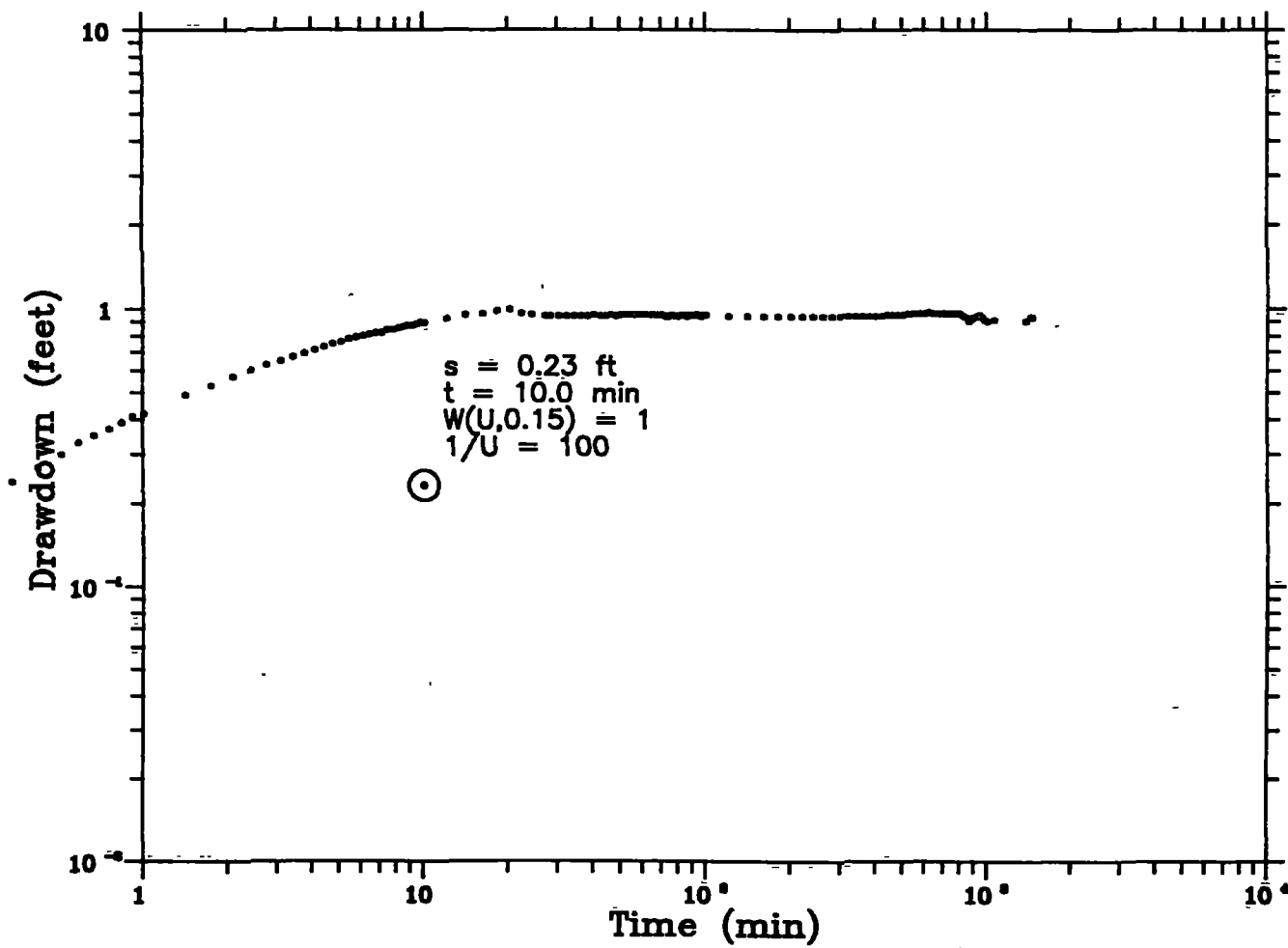


Figure 7. Platteville Aquifer Source Control Well Aquifer Test,
Drawdown Curve for Observation Well W18

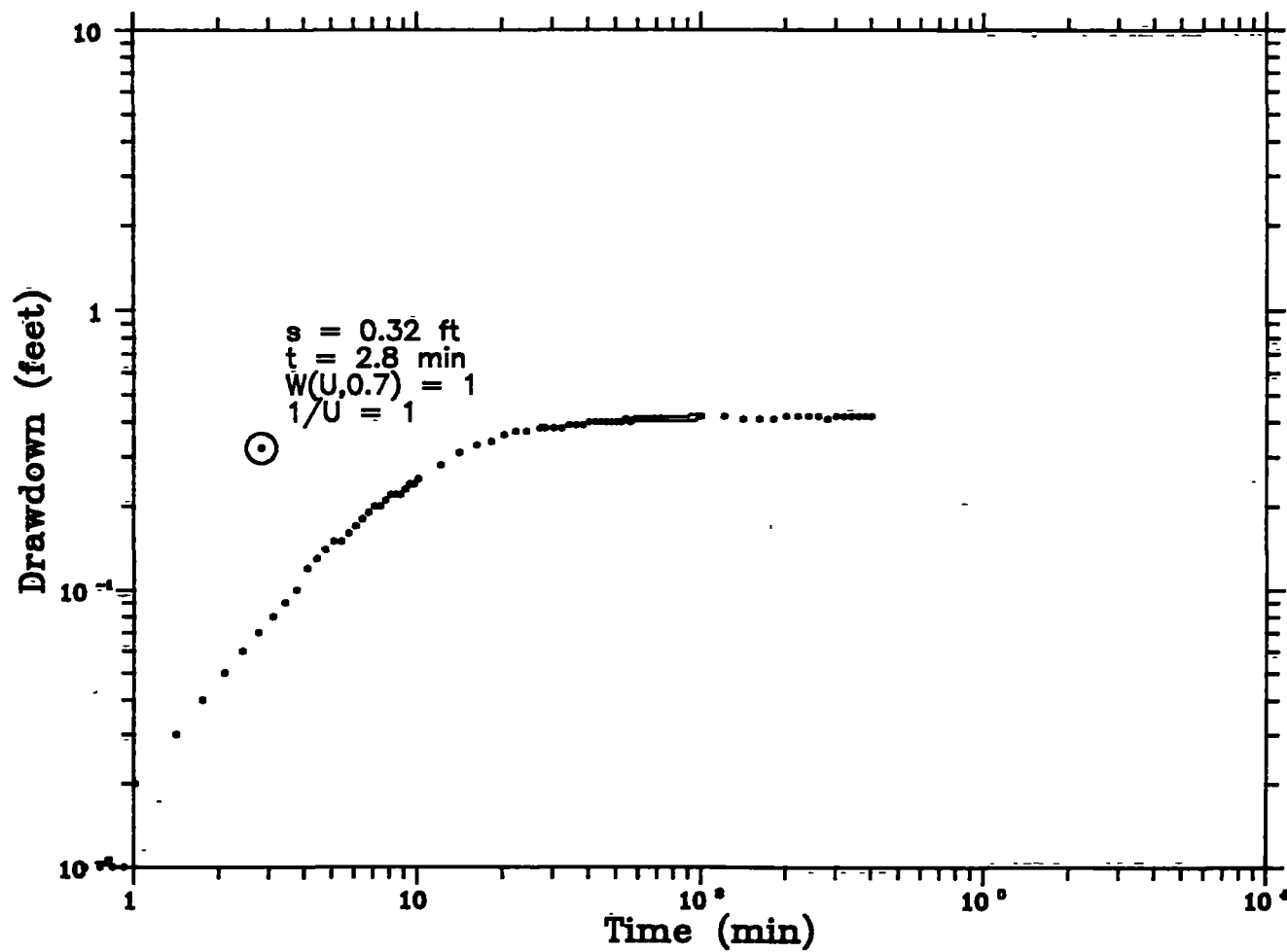


Figure 8. Platteville Aquifer Source Control Well Aquifer Test,
Drawdown Curve for Observation Well W19

TABLE 2

TRANSMISSIVITY VALUES FOR THE
PLATTEVILLE AQUIFER SOURCE CONTROL WELL AQUIFER TEST

<u>WELL NO.</u>	<u>TRANSMISSIVITY (GPD/FT)</u>	<u>STORATIVITY</u>
W19	2.0×10^4	1.08×10^{-5}
W22	4.4×10^4	4.89×10^{-5}
W18	2.8×10^4	1.87×10^{-4}
P121	2.5×10^4	1.8×10^{-5}
W26	2.4×10^4	8.8×10^{-5}

Average T = 2.8×10^4 gpd/ft

Average S = 7.05×10^{-5}

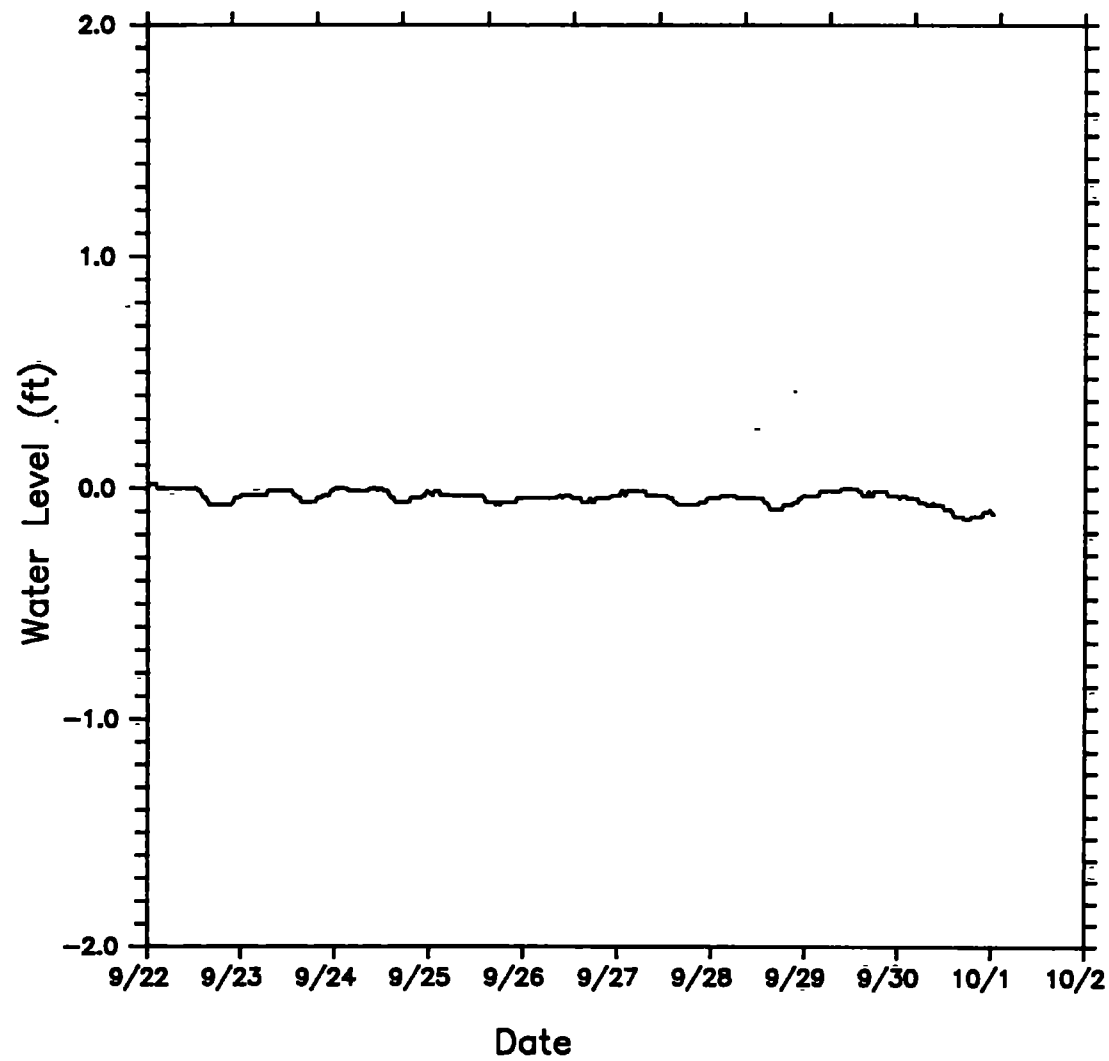


Figure 9. Platteville Aquifer Source Control Well Aquifer Test,
Background Water Level Data - Observation Well W100

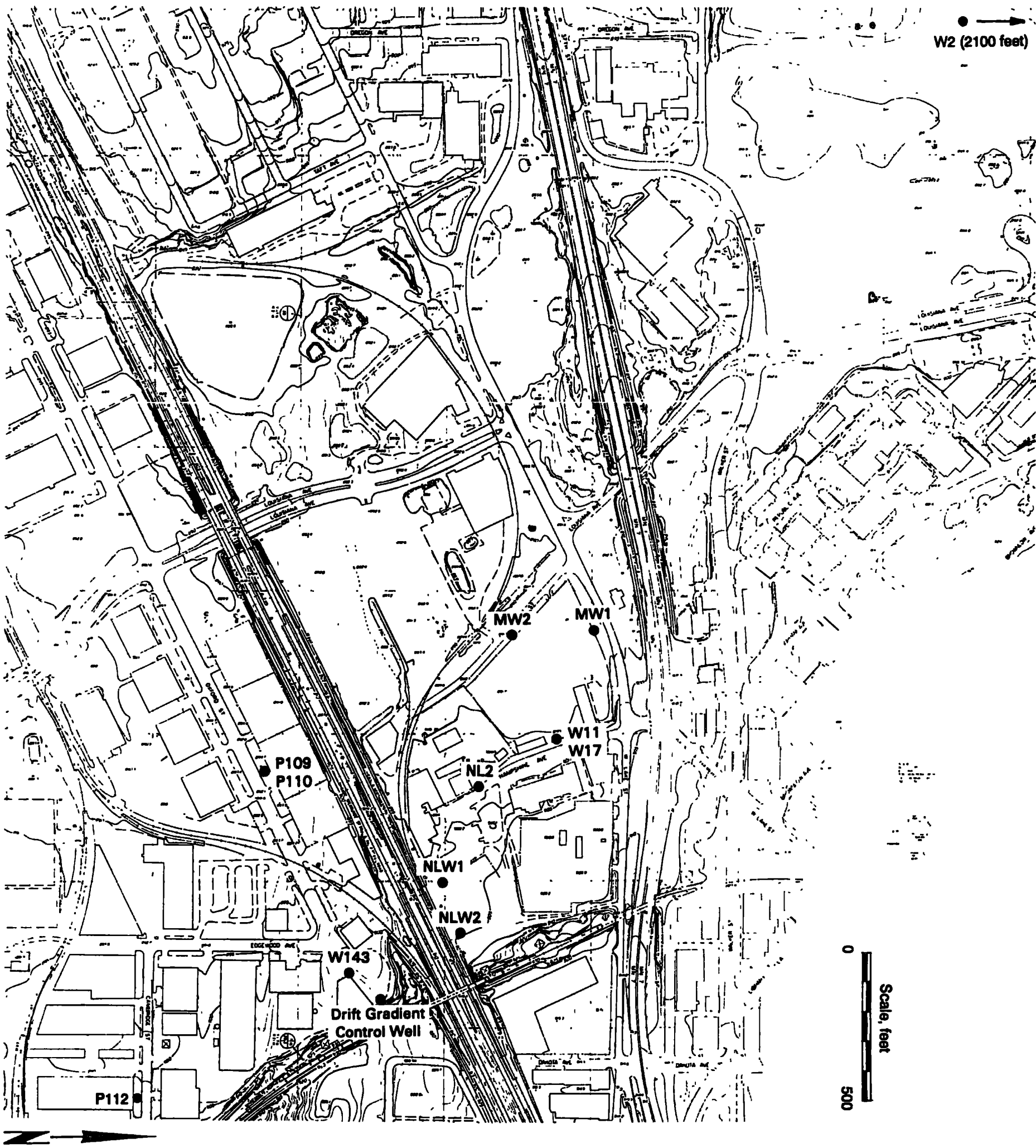


Figure 10. Location of Wells for the Drift Aquifer Gradient Control Well Aquifer Test

levels at the remaining wells. Water levels were recorded for 48 hours prior to the pumping phase of the aquifer test. The Drift-Platteville Aquifer Gradient Control Well was pumped at 90 gallons per minute commencing on October 4, 1987 at 3:40 p.m., and ending on October 8, 1987 at 2:20 p.m. (approximately 95 hours). Higher pumping rates caused too much drawdown in the well. Water levels were then measured for 48 hours to allow for recovery. Data was lost from well W143 at the start of the aquifer test because of an interruption in the transducer circuit.

This aquifer test was different from the first two tests because most of the observation wells did not exhibit water level fluctuations in response to the aquifer test. However, one of the ten observation wells for the third pump test produced data amenable to analysis (W17). The data from this well were analyzed using the Prickett method, with the assumptions presented earlier.

The data and matchpoints used in this analysis are shown in Figure 11. The value calculated for the transmissivity is 5.9×10^{-4} gpd/ft which is within the range of literature values presented for the Drift aquifer and in fair agreement with the results of the Drift source control well pump test above. The value calculated for storativity is 1.06×10^{-2} .

The prepumping data for well W17 did not show any appreciable trends and were not corrected for in the analysis. The background well data (W2) did not show any measurable changes over the duration of the third pump test (Figure 12) and were not corrected in the analysis.

The lack of analyzable data from many of the observation wells for the third pump test is in part due to the lower pumping rate for the pump test (90 gpm), the lack of nearby observation wells that were screened at the same depth as the pumping well, and a lower transmissivity for this portion of the Drift aquifer.

SUMMARY

The Drift-Platteville Aquifer Source and Gradient Control Wells have been installed and tested in accordance with approved plans and schedules and Sections 9.1.2 and 9.2.2. of the RAP. Raw data generated during the three aquifer tests have been transmitted to the Minnesota Pollution Control Agency (Schaefer, 1987). In addition, the data are available for inspection and review at ERT, Inc.'s office in Concord, Massachusetts. Well houses have been

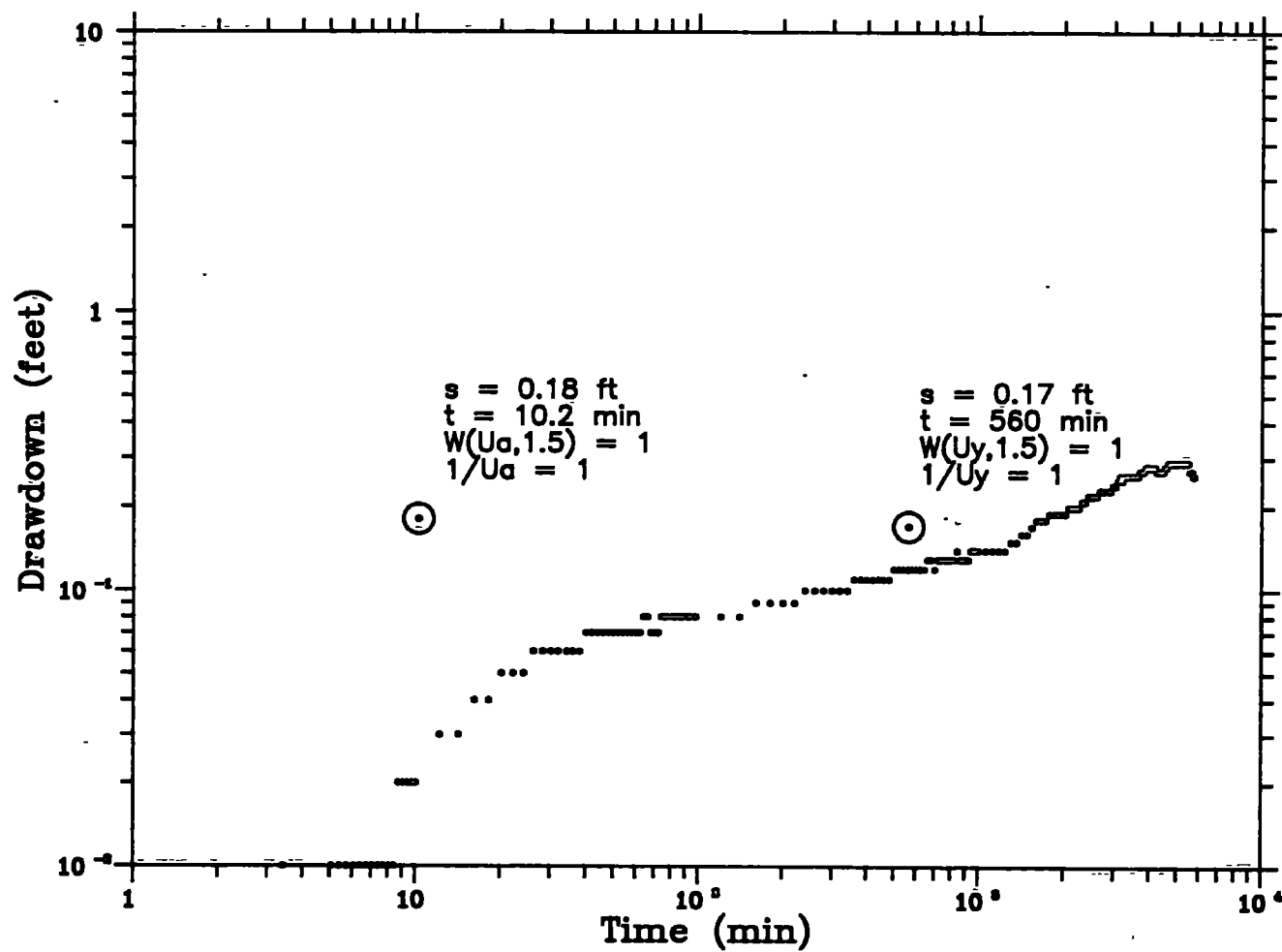


Figure 11. Drift-Platteville Aquifer Gradient Control Well Aquifer Test,
 Drawdown Curve for Observation Well W17

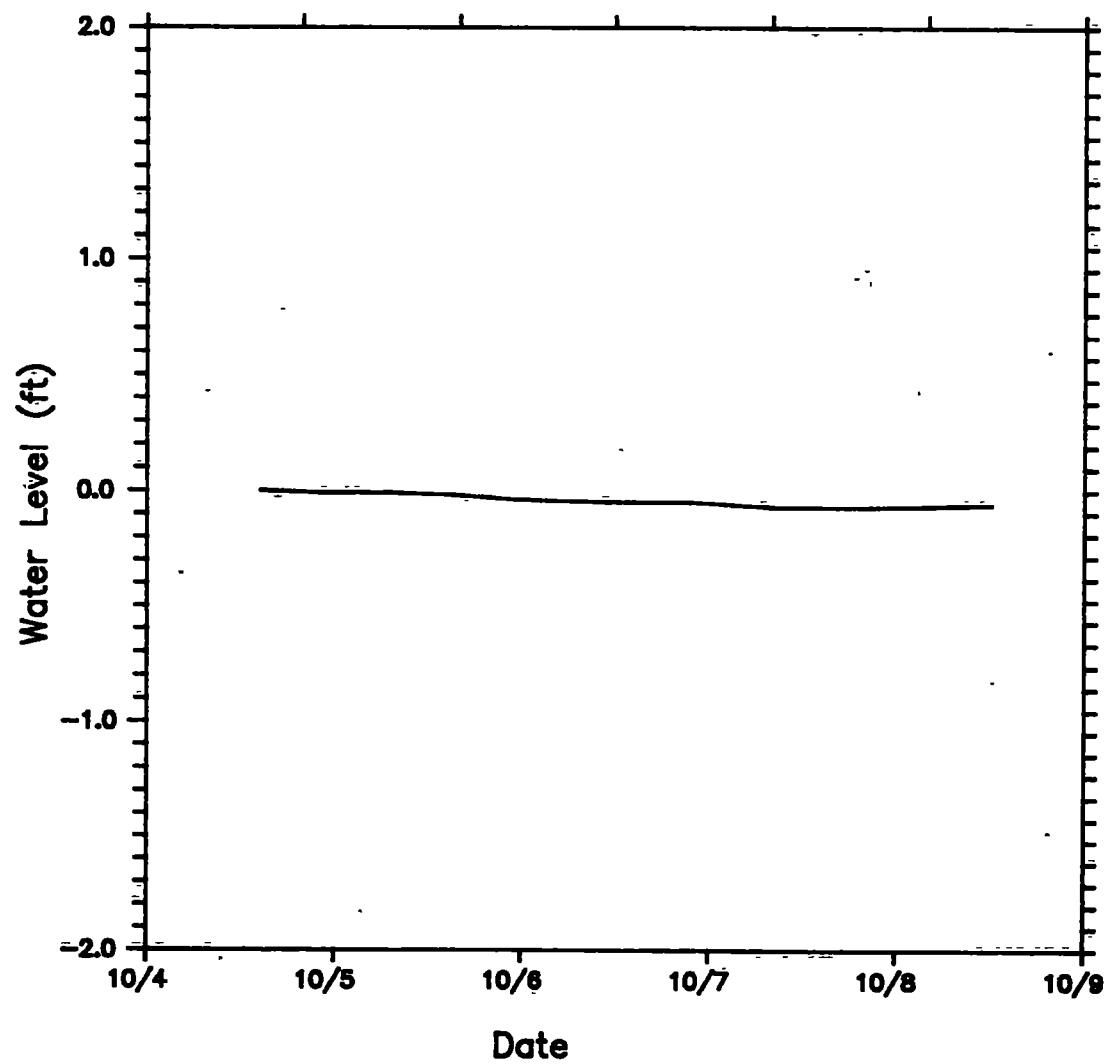


Figure 12. Drift-Platteville Gradient Control Well Aquifer Test,
Background Water Level Data - Observation Well W2

constructed and connections with the sanitary sewer system have been made for the discharge from the pumping wells. The three wells are now ready to commence operation in accordance with the requirements of the RAP.

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APPENDIX A

Drift-Platteville Aquifer Source and Gradient Control

Well Water Well Records, Pilot Soil Boring Logs

and

Grain Size Analyses

DRIFT AQUIFER SOURCE CONTROL WELL

- o **Water Well Record**
- o **Pilot Soil Boring Log**
- o **Grain Size Analyses**



STS Consultants Ltd.

Soil Boring Log, Drift Aquifer Source Control Well

PROJECT NAME

Proposed Well House

ARCHITECT-ENGINEER

ERT

SITE LOCATION

St. Louis Park, Minnesota

DEPTH in feet ELEVATION	SAMPLE NO	SAMPLE TYPE	SAMPLE DISTANCE	RECOVERY	DESCRIPTION OF MATERIAL	UNIT DRY WT. LBS/FT ³	UNCONFINED COMPRESSIVE STRENGTH TONS/FT ²				
							1	2	3	4	5
							PLASTIC LIMIT %				
							WATER CONTENT %				
							LIQUID LIMIT %				
							STANDARD PENETRATION				
							BLOWS FT				
							10	20	30	40	50
	1	SS			Silty fine to medium sand, trace coarse sand and roots - dark brown to black - moist - loose - (SM) - (fill)		5				
	2	SS			Fine to coarse sand, little gravel, little silt - brown - wet to saturated - medium dense - (SM) - (fill)		12				
5.0	3	SS									
	3A	SS					4				
	4	SS			Fine fibrous peat - black - (Pt)		5				
10.0	5	SS			Amorphous peat - black - (Pt)		6				
15.0	6	SS			Organic silty clay, trace fibers - white - firm - (OH)		4				
	6A	SS									
20.0	7	SS			Fine to coarse sand, trace gravel and silt - gray - saturated - medium dense - (SP) Note: Cresote like odor in sample 7 and 8.		10				
	7A	SS									
25.0	8	SS					10				
					End of boring at 26.0 ft. Hollow stem auger to full depth.		* - calibrated penetrometer				

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES IN SITU, THE TRANSITION MAY BE GRADUAL.

WL	2.3 ft.	WS	WS OR WD	BORING STARTED	9/29/86	STS OFFICE	Minnesota
WL	BCR	ACR		BORING COMPLETED	9/29/86	DRAWN BY	KC
WL	1.5 ft.	AB		RIG	CME-45 FOREMAN GD	APP'D BY	WBT
						SHEET NO.	1 OF 1
						STS JOB NO.	94026



STS Consultants Ltd.

PROJECT NAME

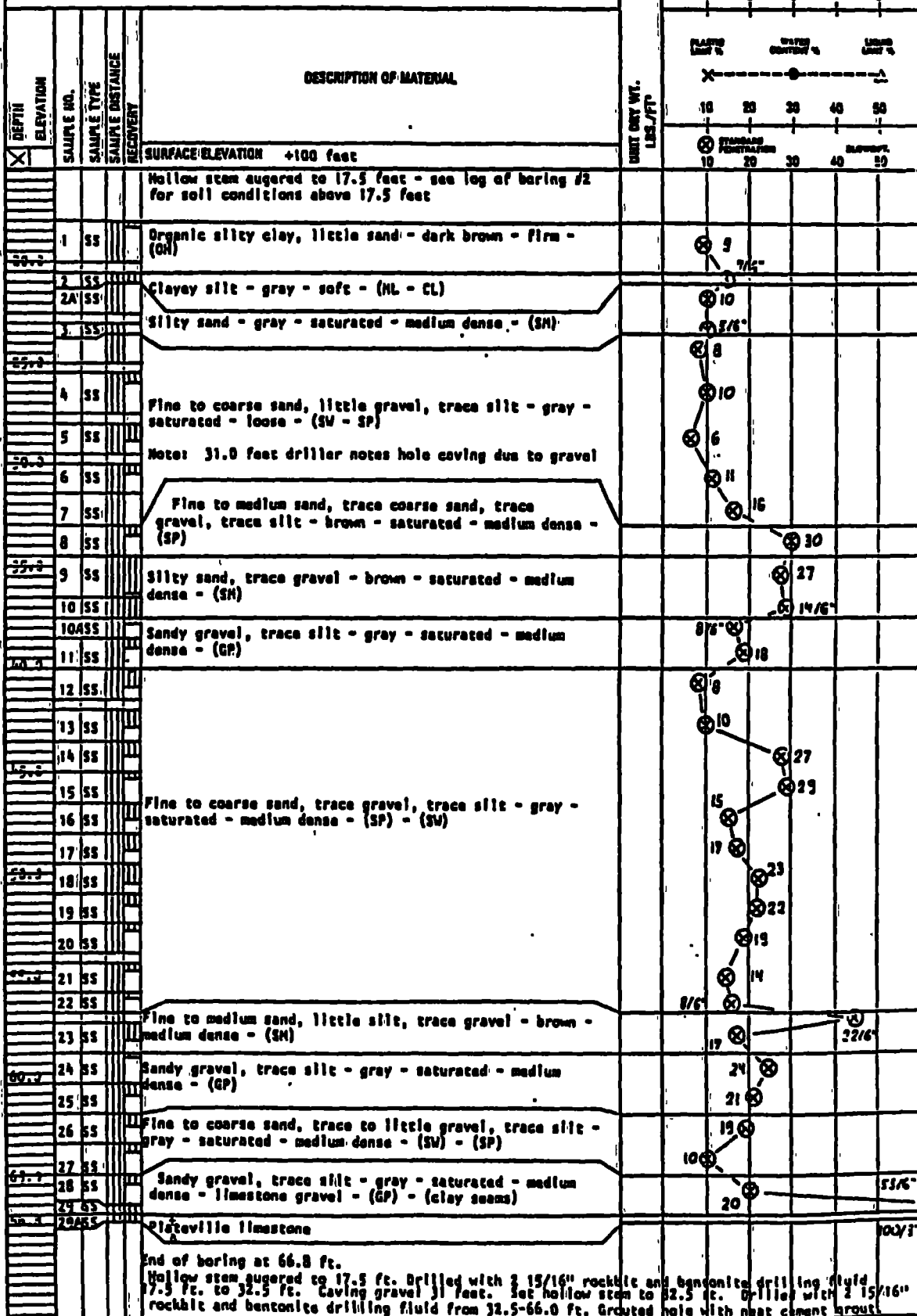
Proposed Wall House

ARCHITECT-ENGINEER

ERT

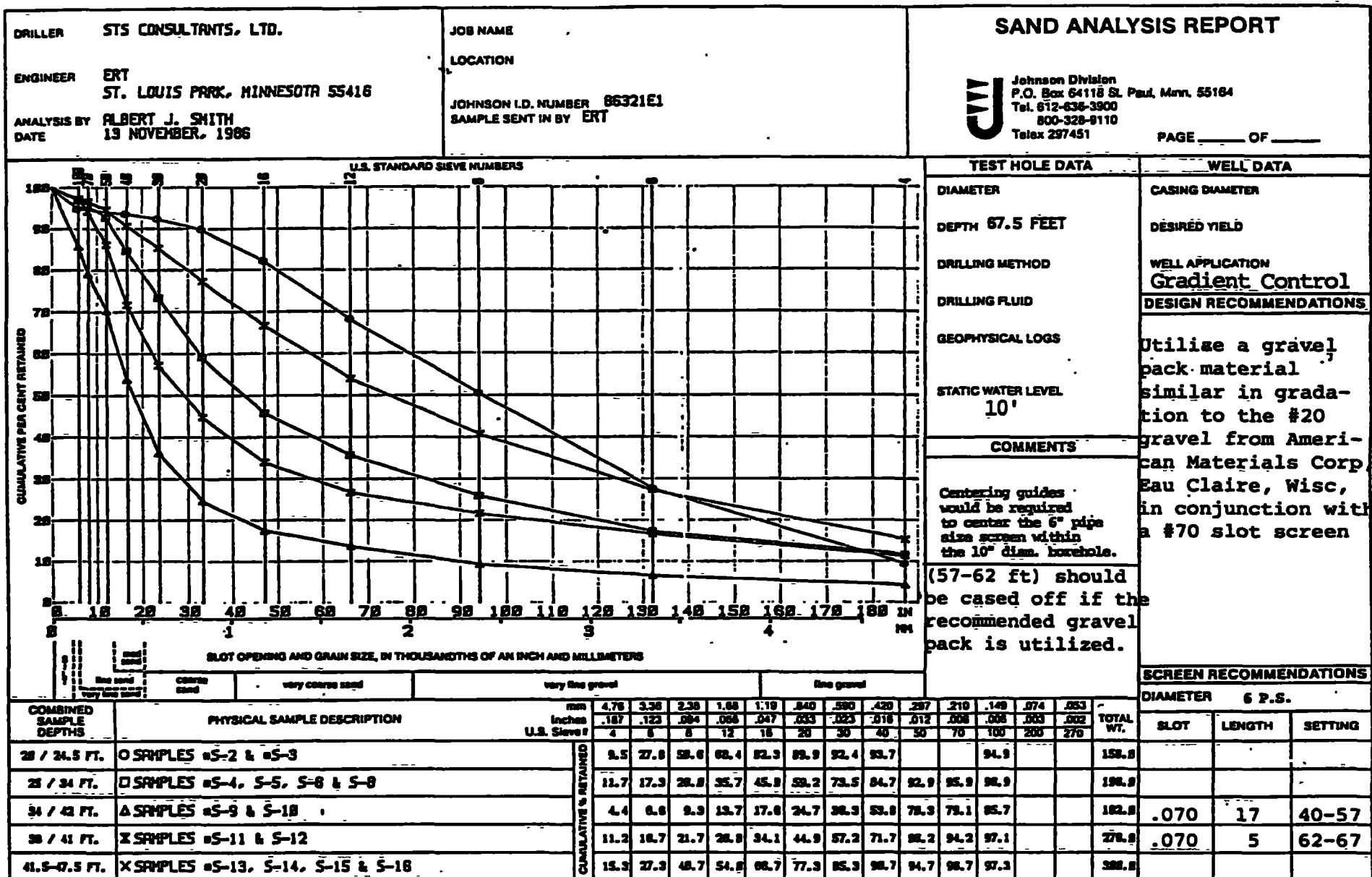
SITE LOCATION

St. Louis Park, Minnesota



THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN SOME THE TRANSITION MAY BE GRADUAL.

WL	WS OR WD	BORING STARTED	10/24/86	STS OFFICE	Minnesota
WL	SCR	ACR	BORING COMPLETED	10/24/86	DRAWN BY AN SHEET NO. 1 OF 1
WL		RIG	CHE-75 FOREMAN GD	APPRO BY SCJ	STS JOB NO. 94026



SO MANY CONSIDERATIONS ENTER INTO THE MAKING OF A GOOD WELL THAT, WHILE WE BELIEVE SLOT SIZES FURNISHED OR RECOMMENDED FROM SAND SAMPLES ARE CORRECT WE ASSUME NO RESPONSIBILITY FOR THE SUCCESSFUL OPERATION OF JOHNSON WELL SCREENS.

DRILLER

ENGINEER

ANALYSIS BY
DATE

13 NOVEMBER, 1986

JOB NAME

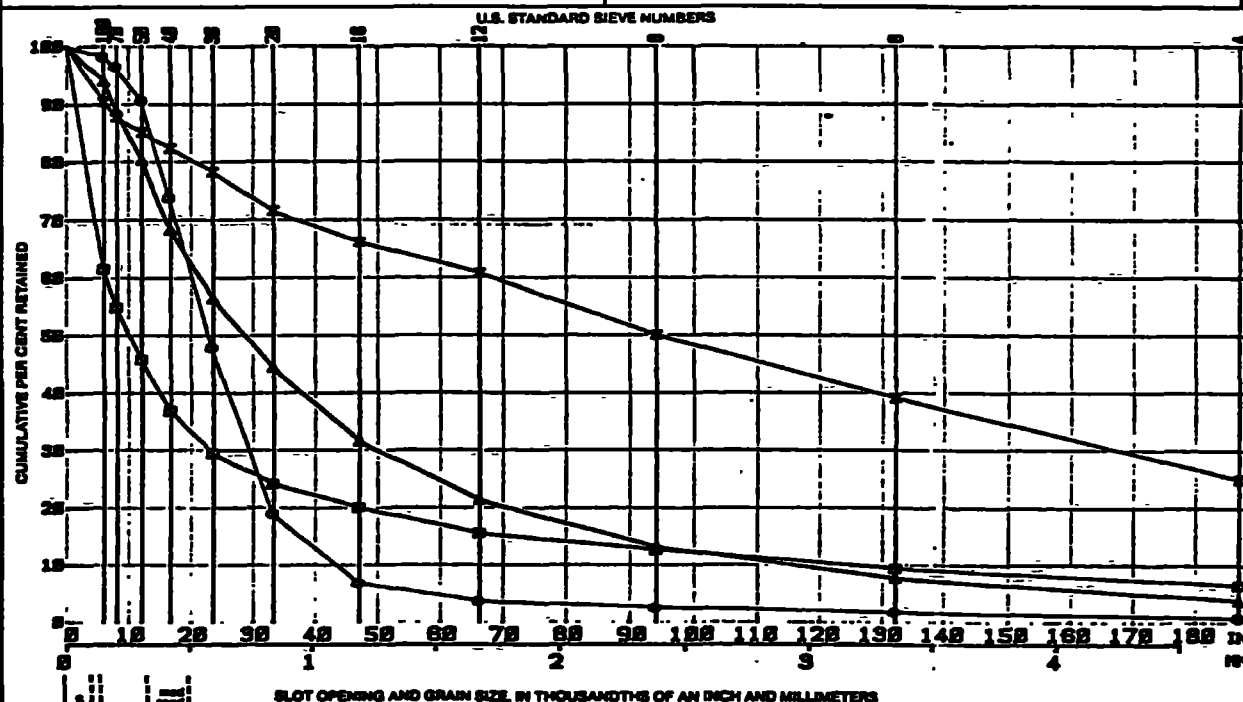
LOCATION

JOHNSON I.D. NUMBER 86321E2
SAMPLE SENT IN BY ERT

SAND ANALYSIS REPORT

Johnson Division
P.O. Box 64118 St. Paul, Minn. 55164
Tel. 612-636-3900
800-328-8110
Telex 297451

PAGE ____ OF ____



TEST HOLE DATA

DIAMETER

DEPTH

DRILLING METHOD

DRILLING FLUID

GEOPHYSICAL LOGS

STATIC WATER LEVEL

WELL DATA

CASING DIAMETER

DESIRED YIELD

WELL APPLICATION

DESIGN RECOMMENDATIONS

COMMENTS

SCREEN RECOMMENDATIONS

DIAMETER

COMBINED SAMPLE DEPTHS	PHYSICAL SAMPLE DESCRIPTION	mm Inches U.S. Sieve #	4.75	3.35	2.35	1.65	1.18	.840	.600	.425	.297	.210	.149	.074	.053	TOTAL WT.	SCREEN RECOMMENDATIONS		
			4	8	16	20	30	40	60	100	200	400	600	1000	2000		SLOT	LENGTH	SETTING
47 - 57 FT.	5 SAMPLES #S-18, S-17, S-19, S-28, S-21, S-22	CUMULATIVE % RETAINED	8.5	1.7	2.6	3.6	6.9	19.1	48.8	74.8	92.8	98.5	99.3			348.8			
57 - 66 FT.	2 SAMPLES #S-23 & S-24		6.6	9.4	12.7	15.6	28.1	24.2	23.5	38.9	45.9	54.9	61.5			244.8			
66 - 68 FT.	3 SAMPLES #S-25, S-26, S-27 & S-28		2.8	7.7	13.2	21.4	31.6	44.4	58.4	68.4	82.3	88.9	94.8			234.8			
68 - 67.5 FT.	1 SAMPLE #S-29		25.8	39.2	58.5	68.8	88.2	71.6	78.4	82.4	85.1	87.8	98.5			148.8			

SO MANY CONSIDERATIONS ENTER INTO THE MAKING OF A GOOD WELL THAT, WHILE WE BELIEVE SLOT SIZES FURNISHED OR RECOMMENDED FROM SAND SAMPLES ARE CORRECT WE ASSUME NO RESPONSIBILITY FOR THE SUCCESSFUL OPERATION OF JOHNSON WELL SCREENS.

PLATTEVILLE AQUIFER SOURCE CONTROL WELL

- o **Water Well Record**

LOCATION OF WELL

WATER WELL RECORD
Minnesota Statutes 156A 01.08MINNESOTA UNIQUE WELL NO
for Water Sample

434044

County Name
Hennepin

Township Name

St. Louis Park

Township Number

117

Range Number

21

Section No

17

Fraction

1/4 1/4 1/4

4 WELL DEPTH (completed)

84

Date of Completion

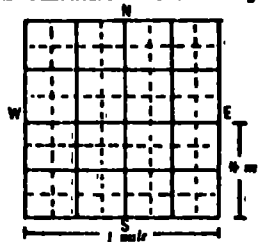
10/12/87

Distance and Direction from Road Intersection or Street Address and City of Well Location

NW Quad Lake Street & Louisiana Ave.

Show exact location of well in section grid with "X"

Sketch map of well location



Addition Name

Block Number

Lot Number

5 DRILLING METHOD

- ☐ Cable tool ☐ Reverse ☐ Driven ☐ Dug
☐ Hollow rod ☐ Air ☐ Bored ☐ _____
☒ Rotary ☐ Jetted ☐ Power t auger

6 DRILLING FLUID

Bentonite

7 USE

- ☐ Domestic ☒ Monitoring ☐ Heat Pump
☐ Irrigation ☐ Public ☐ Industry
☐ Test Well ☐ Municipal ☐ Commercial
☐ Air Conditioning ☐ _____

8 CASING

- ☒ Black ☐ Threaded HEIGHT ☒ Above Surface 2.5 ft
☐ Galv. ☒ Welded Drive Shoe? Yes _____ No ☒ X
☐ Plastic ☐ _____
12 in to 20 ft. Weight 49.56 lbs./ft. 12 in to 20 ft.
6 in to 67 ft. Weight 18.97 lbs./ft. 12 in to 67 ft.
 _____ in to _____ ft. Weight _____ lbs./ft. _____ in to _____ ft.

HOLE DIAM.

9 SCREEN

- Make _____ Or open hole from 67 ft to 84 ft
 Type _____ Dia _____
 Slot/Gauge _____ Length _____ FITTINGS
 Set between _____ ft. and _____ ft.

10 STATIC WATER LEVEL

- 11 ft ☐ below ☒ above land surface Date Measured 8/25/87

11 PUMPING LEVEL (below land surface)

- 40 ft after 48 hrs pumping 50 gpm
 _____ ft after _____ hrs pumping _____ gpm

12 HEAD WELL COMPLETION

- ☐ Pitless adapter manufacturer _____ model _____
☐ Basement offset 30 At least 12" above ground
☐ Plastic casing protection

13 WELL GROUTED?

- ☒ Yes ☐ No
☒ Neat Cement ☒ Bentonite ☐ _____
 Grout material 3% from 0 to 67 ft cu. yds. 1.0

14 NEAREST SOURCES OF POSSIBLE CONTAMINATION

- _____ feet _____ direction _____ type
 Well disinfected upon completion? ☐ Yes ☒ No

15 PUMP

- Date installed 10/12/87 ☐ Not installed
 Manufacturer's name Grundfos
 Model number SP6-10 HP 2 Volts 200
 Length of drop pipe 42 ft. capacity 25 g.p.m.
 Material of drop pipe Steel
 Type ☒ Submersible ☐ L.S. Turbine ☐ Reciprocating
☐ Jet ☐ Centrifugal ☐ _____

16 EXISTING WELLS

- Unused well on property? ☐ Yes ☒ No
 Abandoned ☐ Permanent ☐ Temporary ☐ Not sealed

18 WATER WELL CONTRACTORS CERTIFICATION

- This well was drilled under my jurisdiction and this report is true to the best of my knowledge and belief.

Bergerson-Caswell Inc. 27058

Licensee Business Name

License No

Address 5115 Industrial Street Maple Plain, MNSigned E R Hennrich Date 10/12/87

Authorized Representative

Larry Schmidt

Date 10/12/87

Name of Driller

 5/14 30M
 7/18 30M
 7/18 30M

2 PROPERTY OWNER'S NAME

 City of St. Louis Park
 5005 Minnetonka Blvd.
 St. Louis Park, MN 55416
 Attn: Jim Grube

Address

3 FORMATION LOG

COLOR

HARDNESS OF FORMATION

FROM

TO

Sand & Top Soil

Black

S

0

2

Sand, T. Gravel & Silt

Brown

VS

2

5

Peat

Black

VS

5

15

Silty Clay - Organic

White

S

15

20

Coarse Sand & Gravel

Brown

N

20

35

Coarse Gravel

Multi

M

35

67

Plateville Limestone

Orange-Grn

VH

67

84

Use a second sheet, if needed

17 REMARKS, ELEVATION, SOURCE OF DATA, etc.

87-6

 Wellhouse finished floor elevation=183.93
 Corection to MSL =710.30
 Stick up above floor =1.56
 Measuring point elevation =895.79
 (top of well seal)

 IMPORTANT:
 FILE WITH DFED - WELL OWNER COPY

434044

SEE ATTACHED PAGE 1A 001

DRIFT AQUIFER GRADIENT CONTROL WELL

- o Water Well Record**
- o Pilot Soil Boring Log**
- o Grain Size Analyses**

Date of Completion	10/12/87
--------------------	----------

434043

Bergerson-Caswell Inc. 27058
Lessor Business Name Lessor No
Address 5115 Industrial Street, Maple Plain, MN
Signed *E R Hennrich* Date 10/12/87
Authorized Representative
Glenn Holman
Name of Officer Date 10/12/87

6/74 3041
7/76 3041
7'78 3041

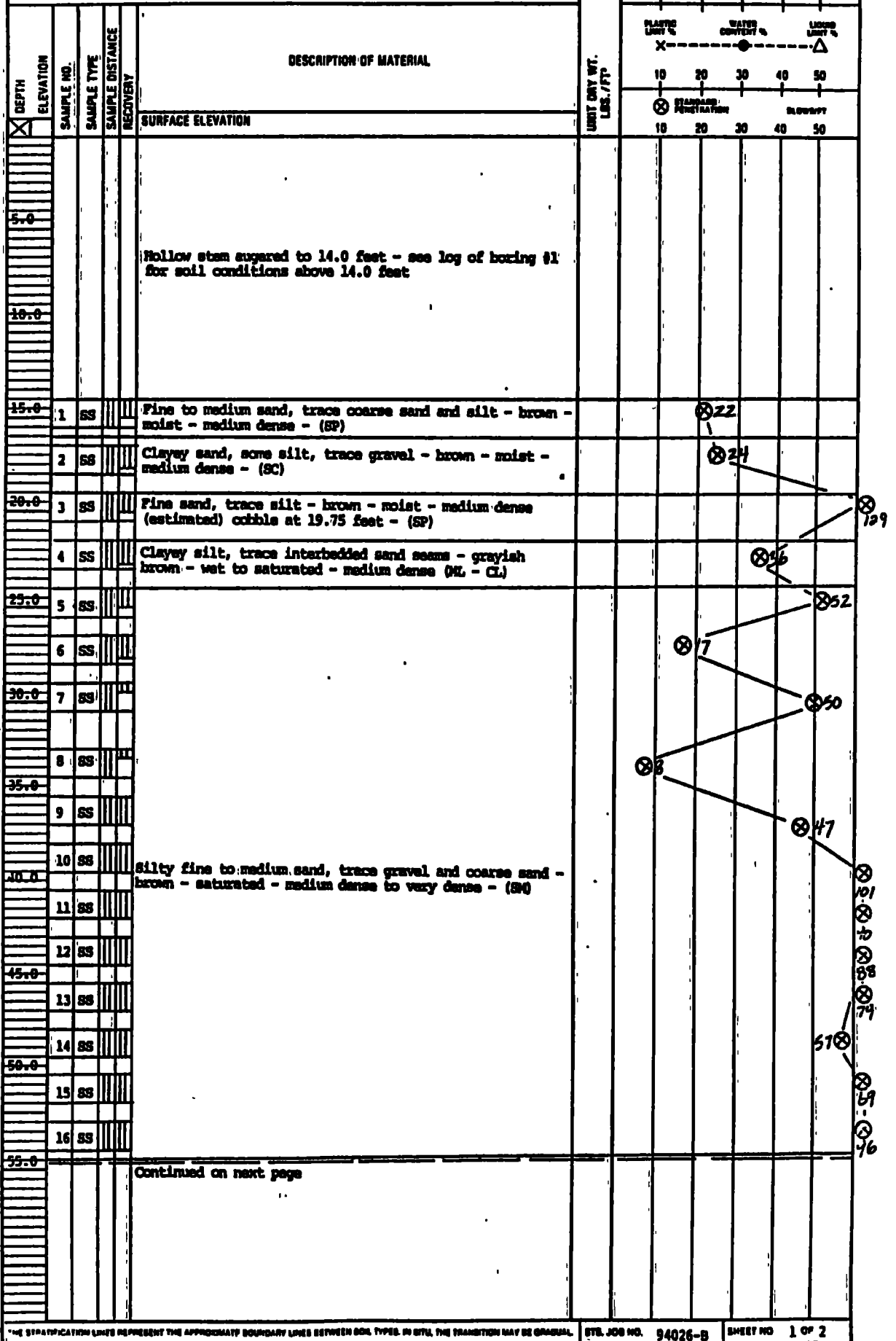


STS Consultants Ltd.

PROJECT NAME
Proposed Wall HouseARCHITECT—ENGINEER
ERT, Inc.

SITE LOCATION

St. Louis Park, Minnesota



THE STRATIFICATION LIMITS REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN SITU, THE TRANSITION MAY BE GRADUAL.

SITE JOB NO. 94026-B

SHEET NO. 1 OF 2



STS Consultants Ltd.

PROJECT NAME

Proposed Wall House

ARCHITECT-ENGINEER

ERT, Inc.

SITE LOCATION

St. Louis Park, Minnesota

DEPTH ELEVATION	SAMPLE NO.	SAMPLE TYPE	SAMPLE DISTANCE	RECOVERY	DESCRIPTION OF MATERIAL	UNIT DRY WT. LBS./FT ³	UNCONFIRMED COMPRESSIVE STRENGTH (lb./sq. ft.)				
							1	2	3	4	5
					SURFACE ELEVATION		PLASTIC LIMIT % WATER CONTENT % LIQUID LIMIT % X-----●-----△ 10 20 30 40 50 STANDARD PENETRATION BLOWCOUNT 10 20 30 40 50				
					Continued from page 1						
55.0	17	SS									
	18	SS									
60.0	19	SS			Medium to coarse sand, trace silt, trace gravel - brownish gray - saturated - extremely dense - (SP - SM)						100 1/4"
	20	SS			Faint chemical odor in sample #20						12 3/8"
65.0	21	SS			Fine to medium sand, trace silt - gray - saturated - extremely dense - (SP - SM)						239
	22	SS									200 1/8"
70.0	23	SS									13 1/8"
	24	SS			Fine sand, trace medium sand and silt - gray - saturated - extremely dense - (SP - SM)						15 7/8"
75.0	25	SS									13 1/8"
	26	SS			Silty sand, trace gravel - brown - wet - extremely dense - (SM)						10 1/8"
80.0					Plateville limestone - inferred						16 1/8"
					End of boring at 80.0 ft. Hollow stem augered to 26.5 ft. Drilled with 2 15/16" rockbit and bentonite drilling fluid from 26.5 - 80.0 ft. Grouted hole with 50 gallons of neat cement grout.						200 1/8"

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN SITU, THE TRANSITION MAY BE GRADUAL.

WL	25.5 ft. NS	WS OR WD	BORING STARTED	3/25/87	STS OFFICE	Minnesota
WL	BCR	ACR	BORING COMPLETED	3/26/87	DRAWN BY	KC SHEET NO. 2 OF 2
WL			RIG	CHE-75 FOREMAN	GD	APP'D BY WBT STS JOB NO. 94026-B

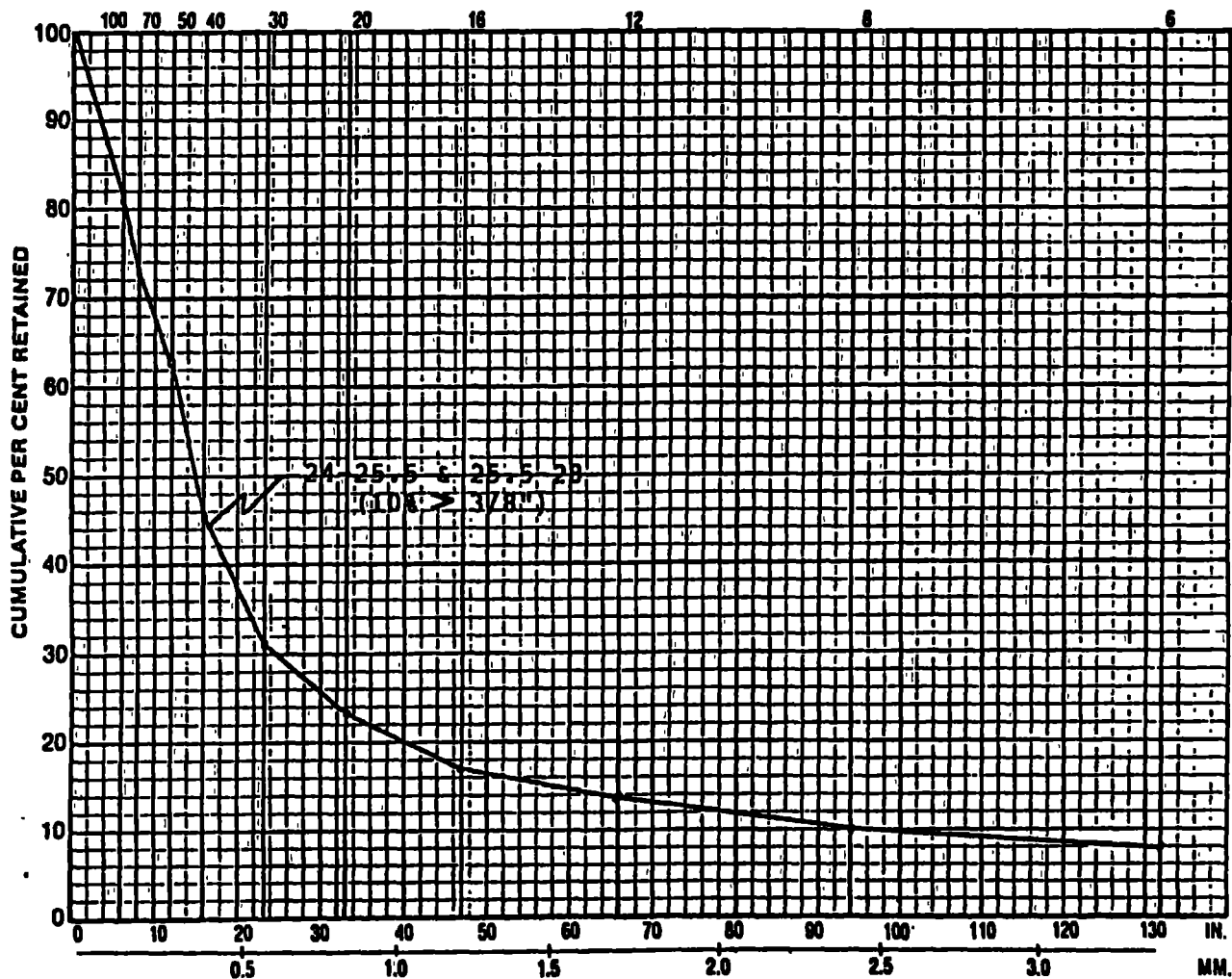


P.O. Box 64118 • St. Paul, MN 55164
 TEL 612-836-3900 or 1-800-328-9991
 TELEX 29-7451
 SEND SAMPLES TO: 1950 OLD HWY 8, NEW BRIGHTON, MN 55112

BY:

Job Name Reilly Tar & Chemical Corporation Date 31 March, 1987
 City St. Louis Park State Minn Zip _____
 Driller STS Consultants, Ltd. Phone _____
 Engineer ERT, St. Louis Park, MN SWL=20 ft. Phone _____
 Remarks Did not test 21.5-23, sandy clay (Gradient control well)

U.S. STANDARD SIEVE NUMBERS



SLOT OPENING AND GRAIN SIZE IN THOUSANDTHS OF AN INCH AND MM.

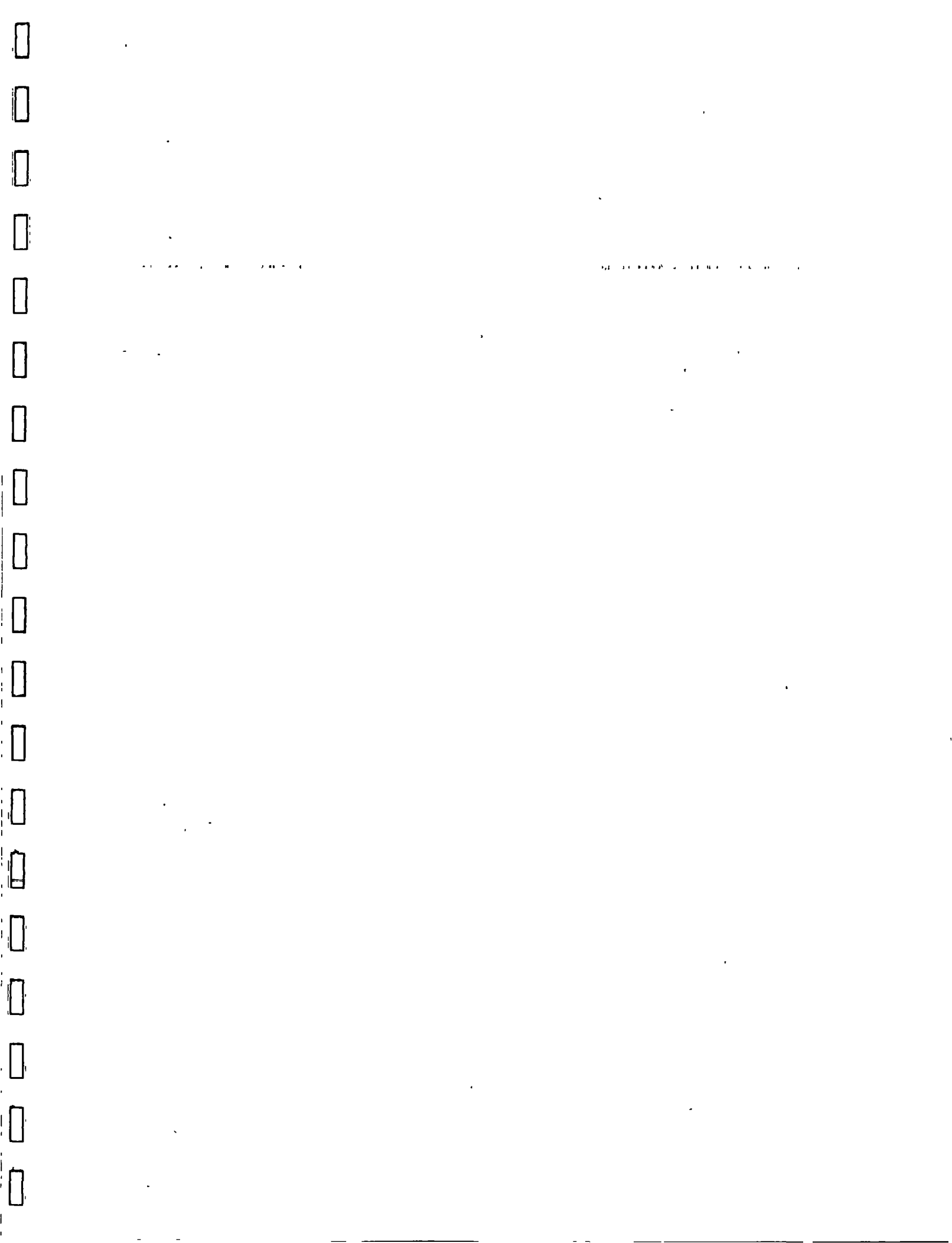
U.S. SIEVE NO.	SLOT OPENING		SAMPLE DEPTHS			
	IN.	MM	21/25			
6	.132	3.38	8			
8	.094	2.38	10			
12	.060	1.68	4			
16	.047	1.19	7			
20	.033	0.84	23			
30	.023	0.60	31			
40	.016	0.42	45			
50	.012	0.30	63			
70	.008	0.21	75			
100	.006	0.15	81			

Comments Utilize a gravel pack material similar in gradation to the #30 gravel from American Materials Corp., Eau Claire, Wisc., in conjunction w/a #40 slot screen.

SCREEN RECOMMENDATIONS: DIAM. _____

SLOT	SETTING	LENGTH
.040	53-78 ft.	25 ft.

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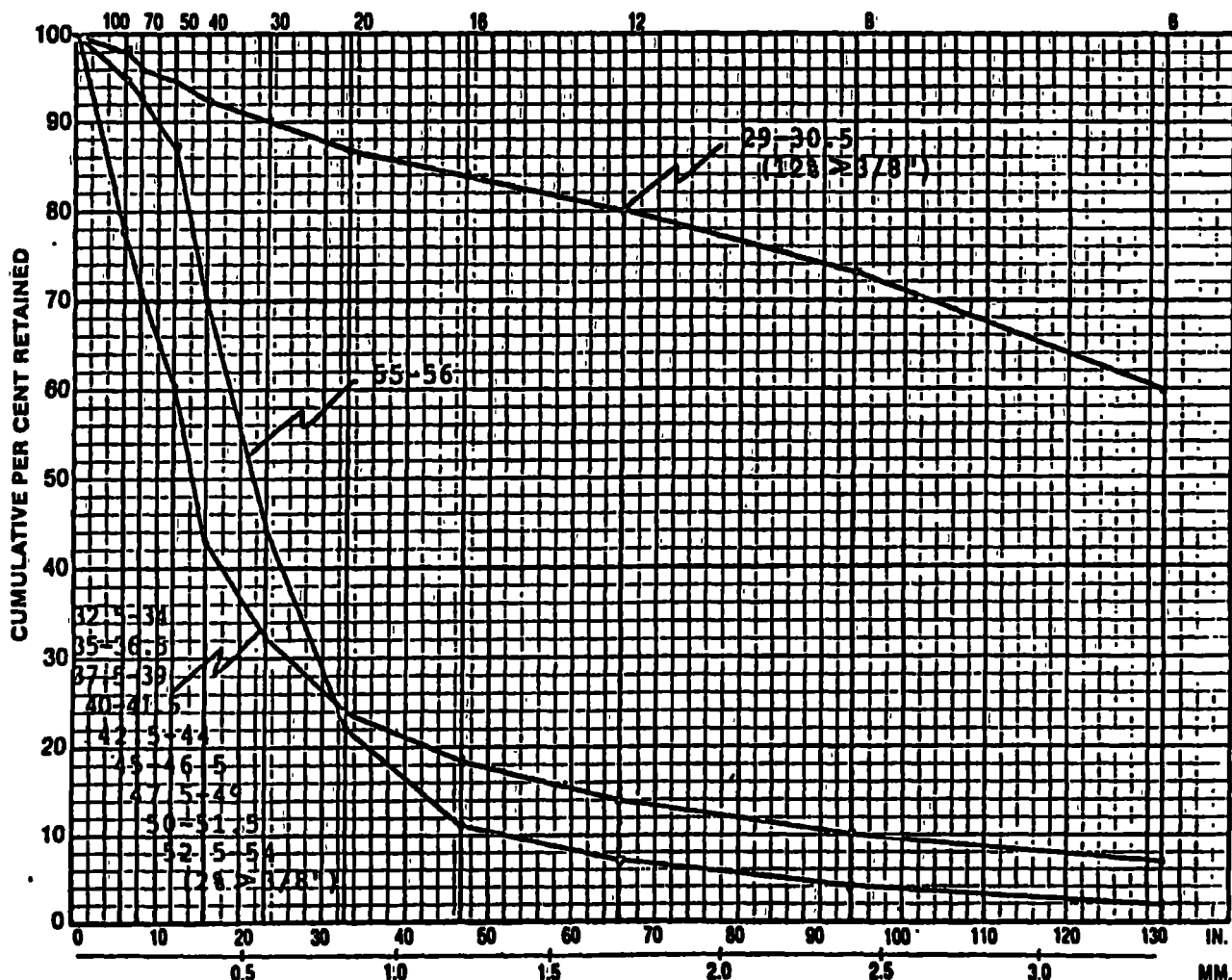
P.O. Box 64118 • St. Paul, MN 55164
TEL 612-838-3900 or 1-800-328-8891
TELEX 29-7451
SEND SAMPLES TO: 1950 OLD HWY 6, NEW BRIGHTON, MN 55112

SAND ANALYSIS

BY:

Job Name _____ Date 31 March, 1987
City _____ State _____ Zip _____
Driller _____ Phone _____
Engineer _____ Phone _____
Remarks _____

U.S. STANDARD SIEVE NUMBERS



SLOT OPENING AND GRAIN SIZE IN THOUSANDTHS OF AN INCH AND MM.

U.S. SIEVE NO.	SLOT OPENING		SAMPLE DEPTHS		
	IN.	MM	29-30-5	32-34	55-56
6	.132	3.36	60	7	2
8	.094	2.38	73	10	4
12	.066	1.68	80	14	7
16	.047	1.19	84	18	11
20	.033	0.84	87	24	22
30	.023	0.60	90	32	44
40	.016	0.42	92	43	70
50	.012	0.30	95	60	87
70	.008	0.21	96	71	93
100	.006	0.15	98	78	95

Comments _____

SCREEN RECOMMENDATIONS: DIAM. _____

SLOT	SETTING	LENGTH

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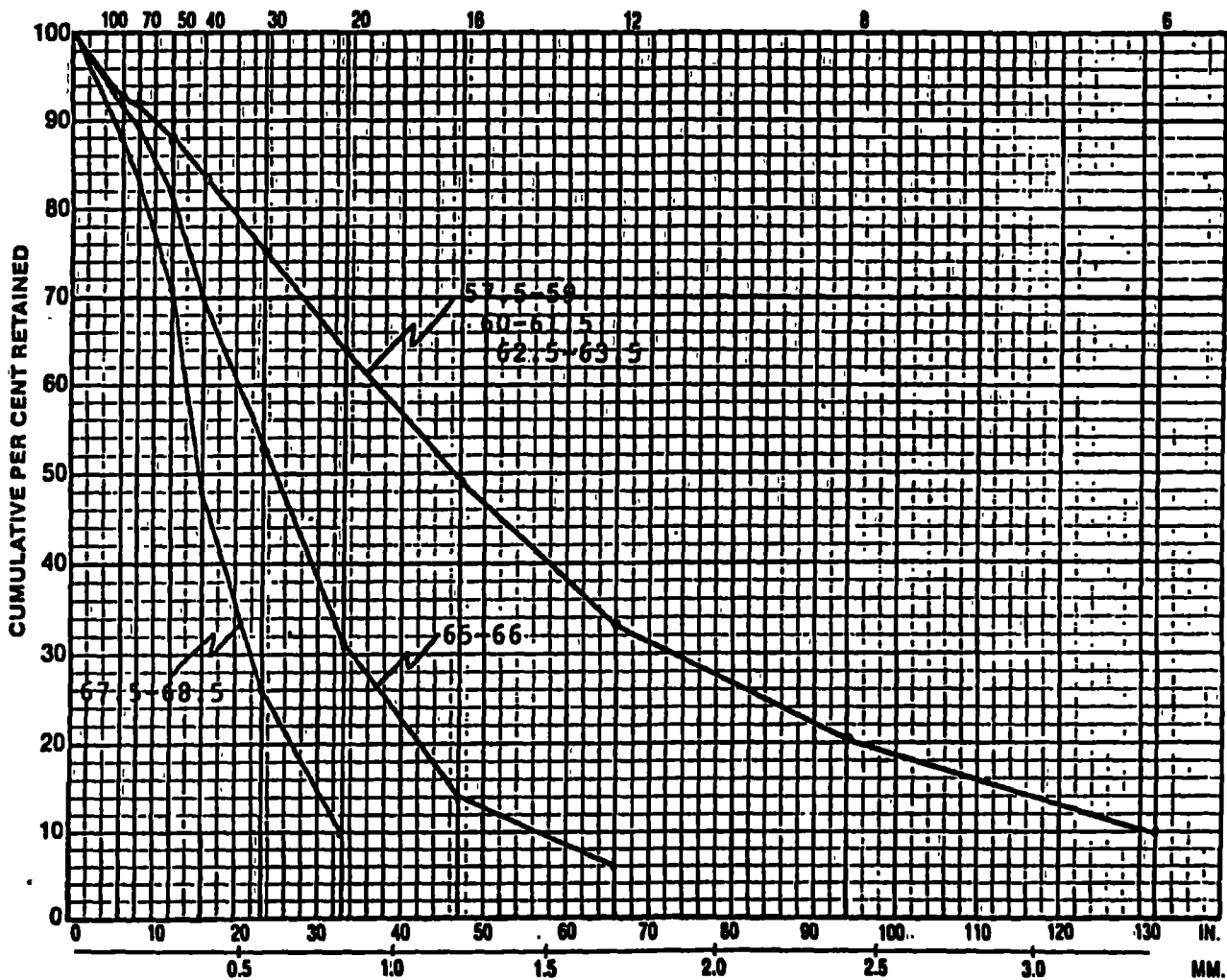


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 TEL. 612-638-3900 or 1-800-328-9891
 TELEX 29-7451
 SEND SAMPLES TO: 1950 OLD HWY 8, NEW BRIGHTON, MN 55112

BY: _____

Job Name _____ Date 31 March, 1987
 City _____ State _____ Zip _____
 Driller _____ Phone _____
 Engineer _____ Phone _____
 Remarks _____

U.S. STANDARD SIEVE NUMBERS



SLOT OPENING AND GRAIN SIZE IN THOUSANDTHS OF AN INCH AND MM.

U.S. SIEVE NO.	SLOT OPENING		SAMPLE DEPTHS		
	IN.	MM	57.5-59	65-66	67.5-68.5
6	.132	3.38	10		
8	.094	2.38	20		
12	.066	1.68	33	6	
16	.047	1.19	49	14	
20	.033	0.84	64	31	10
30	.023	0.60	75	53	26
40	.016	0.42	83	70	48
50	.012	0.30	88	82	71
70	.008	0.21	92	89	83
100	.006	0.15	93	92	88

Comments _____

SCREEN RECOMMENDATIONS: DIAM. _____

SLOT	SETTING	LENGTH

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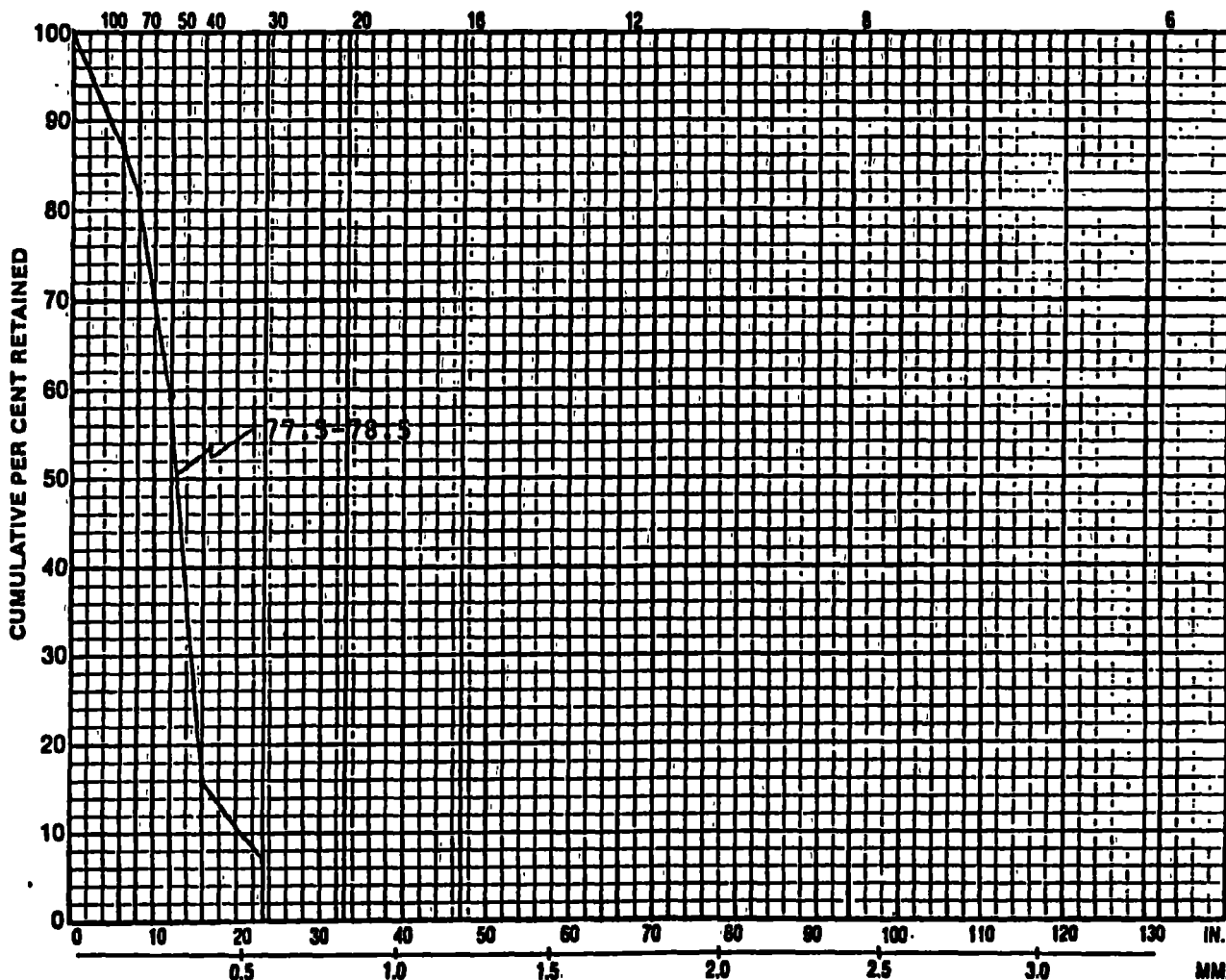


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TEL. 612-636-3900 or 1-800-328-9891
TELEX 29-7451
SEND SAMPLES TO: 1950 OLD HWY. 8, NEW BRIGHTON, MN 55112

BY:

Job Name _____ Date 31 March, 1987
City _____ State _____ Zip _____
Driller _____ Phone _____
Engineer _____ Phone _____
Remarks _____

U.S. STANDARD SIEVE NUMBERS



U.S. SIEVE NO.	SLOT OPENING		SAMPLE DEPTHS	
	IN.	MM		
6	.132	3.36	77.5-85	
8	.094	2.38		
12	.066	1.68		
16	.047	1.19		
20	.033	0.84		
30	.023	0.60	7	
40	.016	0.42	16	
50	.012	0.30	59	
70	.008	0.21	82	
100	.006	0.15	87	

Comments _____

SCREEN RECOMMENDATIONS: DIAM. _____

SLOT	SETTING	LENGTH

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environmental and engineering excellence

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